

ABBREVIATIONS:

ABV	ABOVE
AFF	ABOVE FINISHED FLOOR
BLW	BELOW
BOT	BOTTOM
BOW	BOTTOM OF WALL
CAB	CABINET
CL	CENTERLINE
CONC	CONCRETE
CONT	CONTINUOUS
CP	CENTERPOINT
DET	DETAIL
DIA	DIAMETER
DIM	DIMENSION
DR	DOOR
DS	DOWNSPOUT
D/W	DISHWASHER
EA	EACH
EX	EXISTING
EXT	EXTERIOR
FOC	FACE OF CONCRETE
FOW	FACE OF WALL
FN GRDE	FINISHED GRADE
FNDN	FOUNDATION
FLR	FLOOR
FP	FIREPLACE
GA	GAUGE
GWB	GYPSUM WALL BOARD
HB	HOSE BIBB
HGT	HEIGHT
INFO	INFORMATION
INSUL	INSULATION
INT	INTERIOR
LV	LOW VOLTAGE
MTL	METAL
MFR	MANUFACTURER
N/A	NOT APPLICABLE
NIC	NOT IN CONTRACT
NFC	NOT FOR CONSTRUCTION
OC	ON CENTER
PL	PROPERTY LINE
RAD	RADIUS
RE	REFER TO
SIM	SIMILAR
TBD	TO BE DETERMINED
TG	TEMPERED GLASS
T&G	TONGUE & GROOVE
TOW	TOP OF WALL
TYP	TYPICAL
UNO	UNLESS NOTED OTHERWISE
VIF	VERIFIED IN FIELD
WD	WOOD
WDW	WINDOW

PLAN LEGEND:

	EXISTING WALL TO REMAIN
	NEW FULL-HEIGHT WALL
	NEW FULL-HEIGHT CONCRETE WALL
	PARTIAL-HEIGHT WALL
	PROPERTY LINE
	BUILDING / STRUCTURE ABOVE
	BUILDING / STRUCTURE BELOW
	CENTERLINE
	AREA OF DRAWING REVISION
	ELEVATION MARKER
	SECTION MARKER

GENERAL NOTES:

- DO NOT SCALE DRAWINGS.
- THIS PROJECT SHALL COMPLY WITH ALL GOVERNING REGULATIONS, ORDINANCES, BUILDING CODES, OR COVENANTS OF THE AREA IN WHICH IT IS BUILT.
- APPROVAL BY AN INSPECTOR DOES NOT CONSTITUTE AUTHORITY TO DEVIATE FROM THE DRAWINGS OR SPECIFICATIONS.
- THE CONTRACTOR SHALL SCHEDULE WALK-THROUGHS AT EACH OF BELOW NOTED INTERVALS.
 - PRIOR TO THE COMMENCEMENT OF CONSTRUCTION.
 - PRIOR TO THE COMMENCEMENT OF ALL MECHANICAL + ELECTRICAL WORK.
 - PROVIDE ALL NECESSARY BARRICADES, WARNING SIGNS, + DEVICES TO PROTECT PUBLIC + CONSTRUCTION PERSONNEL DURING CONSTRUCTION.
 - MAINTAIN ALL REQUIRED ACCESS + EGRESS DURING CONSTRUCTION.

DUTY OF COOPERATION:

RELEASE + ACCEPTANCE OF THESE DOCUMENTS INDICATES COOPERATION AMONG THE OWNER, THE CONTRACTOR, + RIPPLE DESIGN STUDIO. ANY ERRORS, OMISSIONS, OR DISCREPANCIES DISCOVERED BY THE USE OF THESE DOCUMENTS SHALL BE REPORTED IMMEDIATELY TO RIPPLE DESIGN STUDIO. FAILURE TO DO SO SHALL RELIEVE RIPPLE DESIGN STUDIO FROM ANY RESPONSIBILITY OF THE CONSEQUENCES.

ANY DEVIATIONS FROM THESE DOCUMENTS WITHOUT THE CONSENT OF RIPPLE DESIGN STUDIO ARE UNAUTHORIZED. FAILURE TO OBSERVE THESE PROCEDURES SHALL RELIEVE RIPPLE DESIGN STUDIO OF RESPONSIBILITY FOR ALL CONSEQUENCES ARISING OUT OF SUCH ACTIONS.

SITE NOTES:

1. ALL IMMEDIATE AREAS AFFECTED BY NEW DEVELOPMENT SHALL BE GRADED AWAY FROM FOUNDATIONS + ADJACENT PROPERTIES @ 10% AS POSSIBLE, 2% MIN.

AVERAGE BUILDING ELEVATION CALCS:

ELEVATION POINT "A"	75.30
SEGMENT LENGTH "A"	30.50
ELEV "A" x SEGMENT "A"	2,296.65
ELEVATION POINT "B"	73.00
SEGMENT LENGTH "B"	15.71
ELEV "B" x SEGMENT "B"	1,146.83
ELEVATION POINT "C"	74.00
SEGMENT LENGTH "C"	31.33
ELEV "C" x SEGMENT "C"	2,318.42
ELEVATION POINT "D"	75.45
SEGMENT LENGTH "D"	3.90
ELEV "D" x SEGMENT "D"	226.35
ELEVATION POINT "E"	73.60
SEGMENT LENGTH "E"	34.25
ELEV "E" x SEGMENT "E"	2,520.80
ELEVATION POINT "F"	64.10
SEGMENT LENGTH "F"	24.70
ELEV "F" x SEGMENT "F"	1,583.27
ELEVATION POINT "G"	65.20
SEGMENT LENGTH "G"	24.00
ELEV "G" x SEGMENT "G"	1,564.80
ELEVATION POINT "H"	65.50
SEGMENT LENGTH "H"	7.75
ELEV "H" x SEGMENT "H"	507.63
ELEVATION POINT "I"	66.00
SEGMENT LENGTH "I"	20.00
ELEV "I" x SEGMENT "I"	1,320.00
ELEVATION POINT "J"	66.70
SEGMENT LENGTH "J"	7.75
ELEV "J" x SEGMENT "J"	516.93
ELEVATION POINT "K"	66.60
SEGMENT LENGTH "K"	14.50
ELEV "K" x SEGMENT "K"	965.70
ELEVATION POINT "L"	66.30
SEGMENT LENGTH "L"	50.00
ELEV "L" x SEGMENT "L"	3,315.00
ELEVATION POINT "M"	64.50
SEGMENT LENGTH "M"	17.00
ELEV "M" x SEGMENT "M"	1,096.50
ELEVATION POINT "N"	66.00
SEGMENT LENGTH "N"	30.50
ELEV "N" x SEGMENT "N"	2,019.00
ELEVATION POINT "O"	66.30
SEGMENT LENGTH "O"	2.50
ELEV "O" x SEGMENT "O"	167.25
ELEVATION POINT "P"	70.00
SEGMENT LENGTH "P"	21.50
ELEV "P" x SEGMENT "P"	1,505.00
TOTAL OF ELEVATION POINTS x SEGMENT LENGHTS	23,064.12
TOTAL SEGMENT LENGTHS	334.99
AVERAGE GRADE	68.85

LOT COVERAGE CALCS:

LOT AREA:	16,678 FT ²
LOT SLOPE:	31.64%
MAXIMUM ALLOWABLE LOT COVERAGE:	(30%) 5,003.4 FT ²
EXISTING RESIDENCE ROOF:	2,773 FT ²
EXISTING VEHICULAR USE:	610 FT ²
TOTAL EXISTING LOT COVERAGE:	3,383 FT ²
EXISTING VEHICULAR TO BE REMOVED:	(148 FT ²)
PROPOSED ADDITIONAL ROOF AREA:	723 FT ²
PROPOSED COVERED PATIO + DECK AREA:	426 FT ²
TOTAL PROPOSED LOT COVERAGE:	1,148 FT ²
TOTAL LOT COVERAGE UPON COMPLETION:	(26.27%) 4,383 FT ²

HARDSCAPE CALCULATIONS:

LOT AREA:	16,678 FT ²
MAXIMUM ALLOWABLE HARDSCAPE COVERAGE:	(12.72%) 2,121.42 FT ²
(9% OF LOT 1501 + BORROWED AREA 620.4)	
EXISTING UNCOVERED DECK:	654 FT ²
EXISTING UNCOVERED PATIO:	125 FT ²
EXISTING WALKWAYS:	343 FT ²
EXISTING STAIRS:	442 FT ²
EXISTING ROCKERIES + RETAINING WALLS:	636 FT ²
TOTAL EXISTING IMPERVIOUS SURFACE:	2,200 FT ²
TOTAL HARDSCAPE TO BE REMOVED:	[909 FT ²]
PROPOSED UNCOVERED DECK AREA:	291 FT ²
PROPOSED WALKWAY AREA:	161 FT ²
TOTAL PROPOSED HARDSCAPE AREA:	452 FT ²
TOTAL HARDSCAPE AREA UPON COMPLETION:	(10.45%) 1,743 FT ²
TOTAL NET HARD SURFACE (IMPERVIOUS) CHANGE:	+543 FT²



REPLACEMENT TREE SCHEDULE:

PLANTING TYPE	COMMON NAME	SCIENTIFIC NAME	SIZE	SPACING	QUANTITY	PLANTING NOTES
TYPE 1	VINE MAPLE	ACER CIRCINATUM	6' TALL MIN.	9'-0"	2	PART SHADE - SHADE, DRY - MOIST

TOTAL REPLACEMENT TREES 2

A NFPA 13D FIRE SPRINKLER SYSTEM IN COMPLIANCE WITH NFPA 13D AND CoMI STANDARDS SHALL BE INSTALLED THROUGHOUT THE RESIDENCE. A SEPARATE FIRE PERMIT IS REQUIRED.

NOTE THAT THIS SPRINKLER SYSTEM REQUIRES A MINIMUM OF 1" WATER METER AND SUPPLY LINE.

FLOOR AREAS:

LOT AREA:	16,678 FT ²
MAXIMUM ALLOWABLE GFA (R-15, 12,000 OR 40%)	(40%) 6,671.20 FT ²
EX BASEMENT FLOOR AREA:	1,193 FT ²
EX BASEMENT 'GARAGE' AREA:	495 FT ²
NEW BASEMENT FLOOR AREA:	751 FT ²
NEW BASEMENT 'EXTERIOR' STORAGE AREA:	[163 FT ²]
TOTAL BASEMENT FLOOR AREA UPON COMPLETION:	2,603 FT ²
EX FIRST FLOOR AREA:	1,881 FT ²
NEW FIRST FLOOR AREA:	494 FT ²
NEW FIRST FLOOR COVERED DECK:	248 FT ²
TOTAL FIRST FLOOR AREA UPON COMPLETION:	2,623 FT ²
TOTAL FLOOR AREA UPON COMPLETION:	5,226 FT ²
EXISTING GARAGE FLOOR AREA:	384 FT ²
PROPOSED GARAGE FLOOR AREA:	270 FT ²
TOTAL GARAGE AREA UPON COMPLETION:	654 FT ²
TOTAL GROSS FLOOR AREA UPON COMPLETION:	(35.2%) 5,879 FT ²

MINIMUM SIDE YARD WIDTH:

LOT AREA:	17,123 FT ²
EFFECTIVE LOT WIDTH:	125'-7"
REQUIRED COMBINED SIDE YARD:	(17%) 21.35'
MINIMUM SIDE YARD BASED ON LOT WIDTH:	(33% OF 21.35') 7.04'

TREE CALCULATIONS:

EXISTING TREES:	DBH:	CONDITION:	RETAINED?	MINCC 1916 LIST? (REGULATED)	EXCEPTION?	REPLACEMENT TREES
TREE #63: CARPINUS BETULUS	6.0 IN	GOOD	YES	NO	NO	0
TREE #64: PRUNUS LUSTANICA	11.0 IN	GOOD	YES	YES	NO	0
TREE #65: PRUNUS LUSTANICA	8.0 IN	GOOD	YES	NO	NO	0
TREE #66: PRUNUS LUSTANICA	8.0 IN	FAIR	YES	NO	NO	0
TREE #67: THUJA PLICATA	22.0 IN	FAIR	YES	YES	NO	0
TREE #68: SALIX SITCHENSIS	6.0 IN	POOR	YES	NO	NO	0
TREE #69: THUJA PLICATA	22.0 IN	FAIR	YES	YES	NO	0
TREE #70: THUJA PLICATA	19.0 IN	FAIR	YES	YES	NO	0
TREE #71: THUJA PLICATA	20.0 IN	FAIR	YES	YES	NO	0
TREE #72: THUJA PLICATA	20.0 IN	GOOD	YES	YES	NO	0
TREE #73: PSEUDOTSUGA MENZIESII	18.0 IN	GOOD	YES	YES	NO	0
TREE #74: PSEUDOTSUGA MENZIESII	17.0 IN	FAIR	YES	YES	NO	0
TREE #75: PICEA SITCHENSIS	12.0 IN	FAIR	YES	YES	YES	0
TREE #76: PSEUDOTSUGA MENZIESII	42.0 IN	GOOD	YES	YES	YES	0
TREE #77: THUJA PLICATA	44.0 IN	FAIR	YES	YES	YES	0
TREE #78: CORNUS NUTTALLII	12.0 IN	FAIR	YES	YES	YES	0
TREE #79: CORNUS NUTTALLII	7.0 IN	FAIR	YES	YES	YES	0
TREE #80: CORNUS NUTTALLII	7.0 IN	POOR	YES	YES	YES	0
TREE #81: ARBUTUS ANDRACHNE	15.0 IN	FAIR	YES	YES	NO	0
TREE #82: ARBUTUS ANDRACHNE	10.0 IN	FAIR	YES	YES	NO	0
TREE #83: PINUS MONTICOLA	19.0 IN	FAIR	YES	YES	NO	0
TREE #84: ARBUTUS ANDRACHNE	15.0 IN	FAIR	YES	YES	NO	0
TREE #85: ARBUTUS ANDRACHNE	16.0 IN	FAIR	YES	YES	NO	0
TREE #86: PRUNUS LUSTANICA	11.0 IN	GOOD	NO	YES	NO	2
TREE #87: CORNUS NUTTALLII	11.0 IN	FAIR	YES	YES	YES	0

TOTAL EXISTING CALIPER

PROPOSED CALIPER OF TREES TO REMAIN:

336.0 IN

386.0 IN

97.22%

PROJECT INFO:

PROJECT ADDRESS:
5419 96TH AVE SE
MERCER ISLAND, WA 98040

SCOPE OF WORK:
ADDITION AND REMODEL PER PLAN

ZONE:
R-15

LEGAL DESCRIPTION:
LOT 11 C&V H&Ls, ACCORDING TO THE PLAT THEREOF RECORDED IN VOLUME 70 OF PLATS, PAGES 95 AND 96, IN KING COUNTY, WASHINGTON.
SITUATE IN THE COUNTY OF KING, STATE OF WASHINGTON.

ACCESSOR'S PARCEL NUMBER:
143870010

BUILDING CODE + OCCUPANCY:
2021 IRC (ARCHITECTURAL) + IBC (STRUCTURAL)
R-3 SINGLE FAMILY RESIDENTIAL (RESIDENCE)
U STORAGE (GARAGE, STORAGE)

TYPE OF CONSTRUCTION:
TYPE-V-N NON-SPRINKLERED

PROJECT TEAM:

CLIENT / OWNER:
ALPAY RESIDENCE
5419 96TH AVE SE
MERCER ISLAND, WA 98040
206.913.2333

ARCHITECT / APPLICANT:
RIPPLE DESIGN STUDIO - JIM DEARTH
4303 STONE WAY N
SEATTLE, WA 98103
206.913.2333

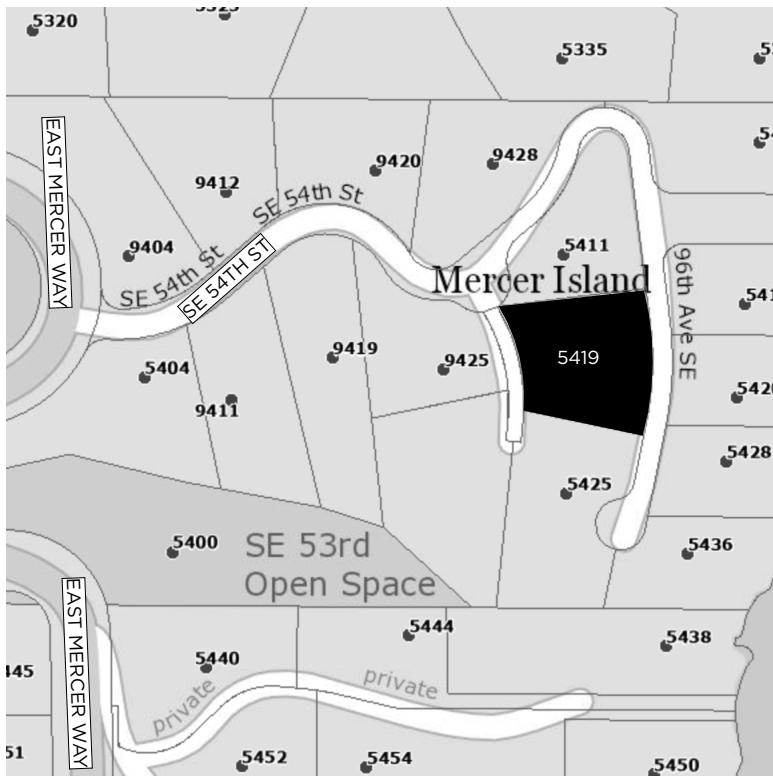
SURVEYOR:
TERRANE
10801 MAIN STREET SUITE 102
BELLEVUE, WA 98004
425.458.4488

GEOTECHNICAL ENGINEER:
GEOTECH CONSULTANTS, INC. - MARC MCGINNIS
2401 10TH AVE E
SEATTLE, WA 98102
425.747.5618

STRUCTURAL ENGINEER:
BUKER ENGINEERING - DANIEL BUKER
4303 STONE WAY N
SEATTLE, WA 98103
206.258.6333

CONTRACTOR:
GT RESIDENTIAL CONTRACTING - GRAHAM TRAVIS
PO BOX 6241
EDMONDS, WA 98026
206.769.4128

VICINITY MAP:



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9790
REGISTERED
ARCHITECT
JIM DEARTH
JAMES M DEARTH
STATE OF WASHINGTON

ALPAY RESIDENCE

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MERCER ISLAND, WA 98040

SITE PLAN

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9 DECEMBER 2025
CAR-2 PERMIT SUBMITTAL
23 DECEMBER 2025
PERMIT RESPONSE
7 JANUARY 2026
PERMIT RESPONSE
26 FEBRUARY 2026

A1.0

ALPAY
AUTHORED: 2/27/24

We are the measure | **terrane.net**

TOPOGRAPHIC & BOUNDARY SURVEY

PARCEL NO. 1438700110
5419 96TH AVE SE

5419 96TH AVENUE SOUTHEAST
MERCER ISLAND, WA 98040



TERRANE

11235 SE 6th St, Suite 130
Bellevue, WA 98004
p: 425-458-4488 | e: info@terrane.net

JOB NUMBER:	250938
DATE:	06/20/25
DRAFTED BY:	CAS
CHECKED BY:	JPS
SCALE:	1"= 10'

REVISION HISTORY

SHEET NUMBER

2 OF 2

SLOPE, SCOUR, BATTER, DISCONTINUITY
 INFORMATION AND EXTENT OF STEEP SLOPES SHOWN ON THIS DRAWING ARE FOR INFORMATIONAL PURPOSES ONLY AND CANNOT BE RELIED ON FOR DESIGN AND/OR CONSTRUCTION. THE PITCH, LOCATION, AND EXTENT ARE BASED SOLELY ON OUR GENERAL OBSERVATIONS ON SITE AND OUR CURSORY REVIEW OF READILY AVAILABLE PUBLIC DOCUMENTS; AS SUCH, TERRACE CANNOT BE LIABLE OR RESPONSIBLE FOR THE ACCURACY OR COMPLETENESS OF ANY STEEP SLOPE INFORMATION. ULTIMATELY, THE LIMITS AND EXTENT OF ANY STEEP SLOPES ASSOCIATED WITH ANY SETBACKS OR DISCONTINUITIES IN CONSTRUCTION SHALL BE DETERMINED AND APPROVED BY THE REVIEWING AGENCY BEFORE ANY CONSTRUCTION CAN OCCUR.



INDEXING INFORMATION

INDEXING INFORMATION

NW ⁴	NE ⁴
SW ⁴	SE ⁴

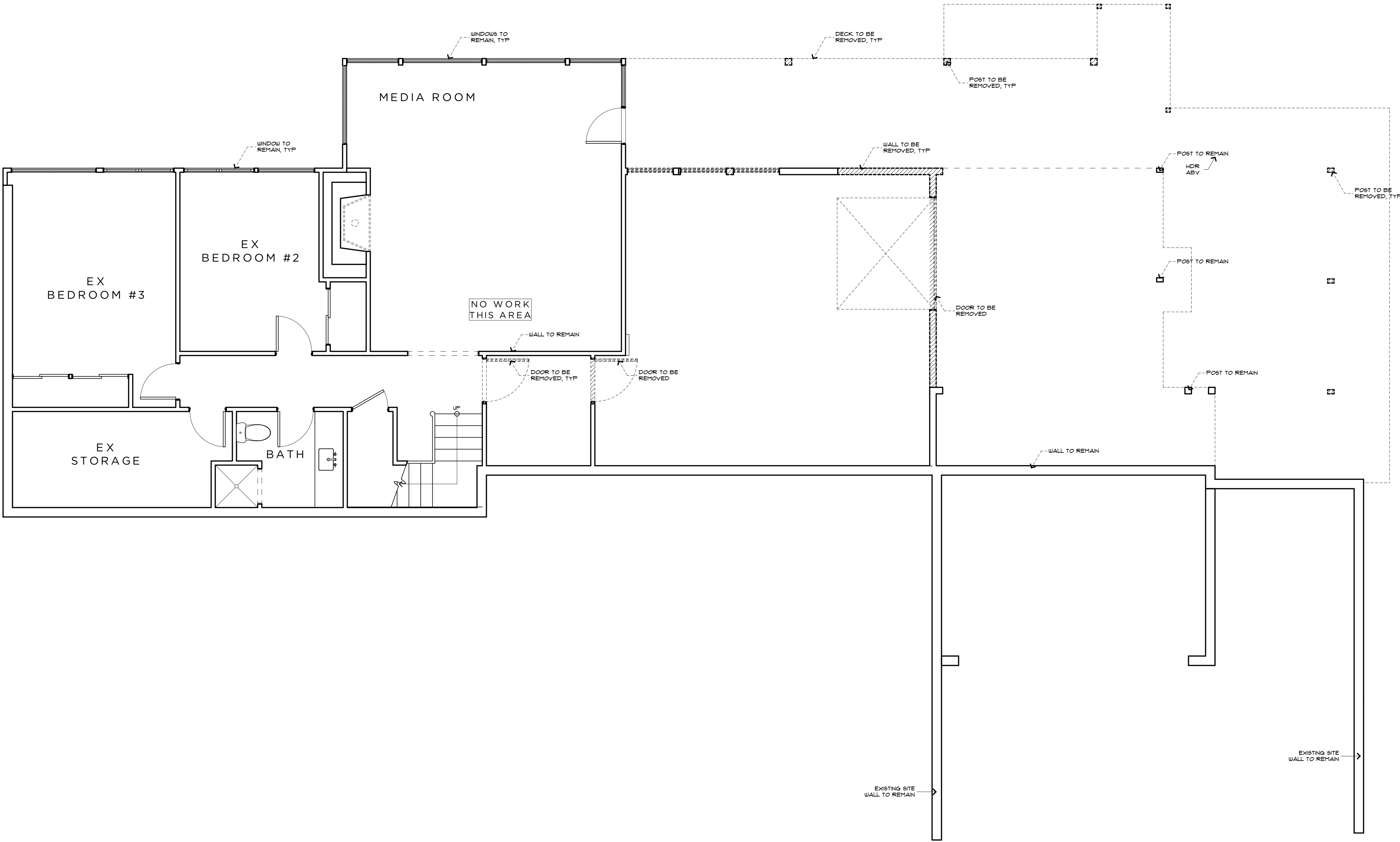
SE 1/4 NE 1/4
SECTION: 19
TOWNSHIP: 24N
RANGE: 05E, W.M.
COUNTY: KING

DEMO LEGEND:

- EXISTING WALL TO REMAIN
- WALL TO BE REMOVED
- EXISTING WINDOW TO REMAIN
- WINDOW TO BE REMOVED
- EXISTING DOOR TO REMAIN
- DOOR TO BE REMOVED
- ROOF TO BE REMOVED
- STAIR/CASEWORK/OTHER TO BE REMOVED

DEMO NOTES:

1. CONTRACTOR TO VERIFY STRUCTURAL BEARING OF ALL ARCHITECTURAL ELEMENTS, INCLUDING WALLS + COLUMNS, + PROVIDE TEMPORARY SHORING PER STRUCTURAL ENGINEER AS REQUIRED PRIOR TO DEMOLITION.
2. ALL DEMOLITION SHALL BE PERFORMED IN SUCH A MANNER TO MINIMIZE COLLATERAL DAMAGE TO SURROUNDING AREAS.
3. SALVAGE ALL EXISTING WINDOWS, DOORS, PLUMBING FIXTURES, FLOORING, TRIM, ETC, IF POSSIBLE. CONFIRM REUSE WITH RIPPLE DESIGN STUDIO + CLIENT PRIOR TO DISPOSAL.



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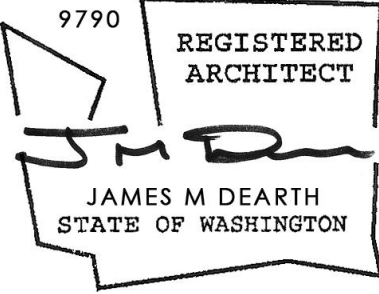
SCALE: 1/4" = 1'-0"



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ALPAY RESIDENCE
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B A S E M E N T
D E M O L I T I O N P L A N

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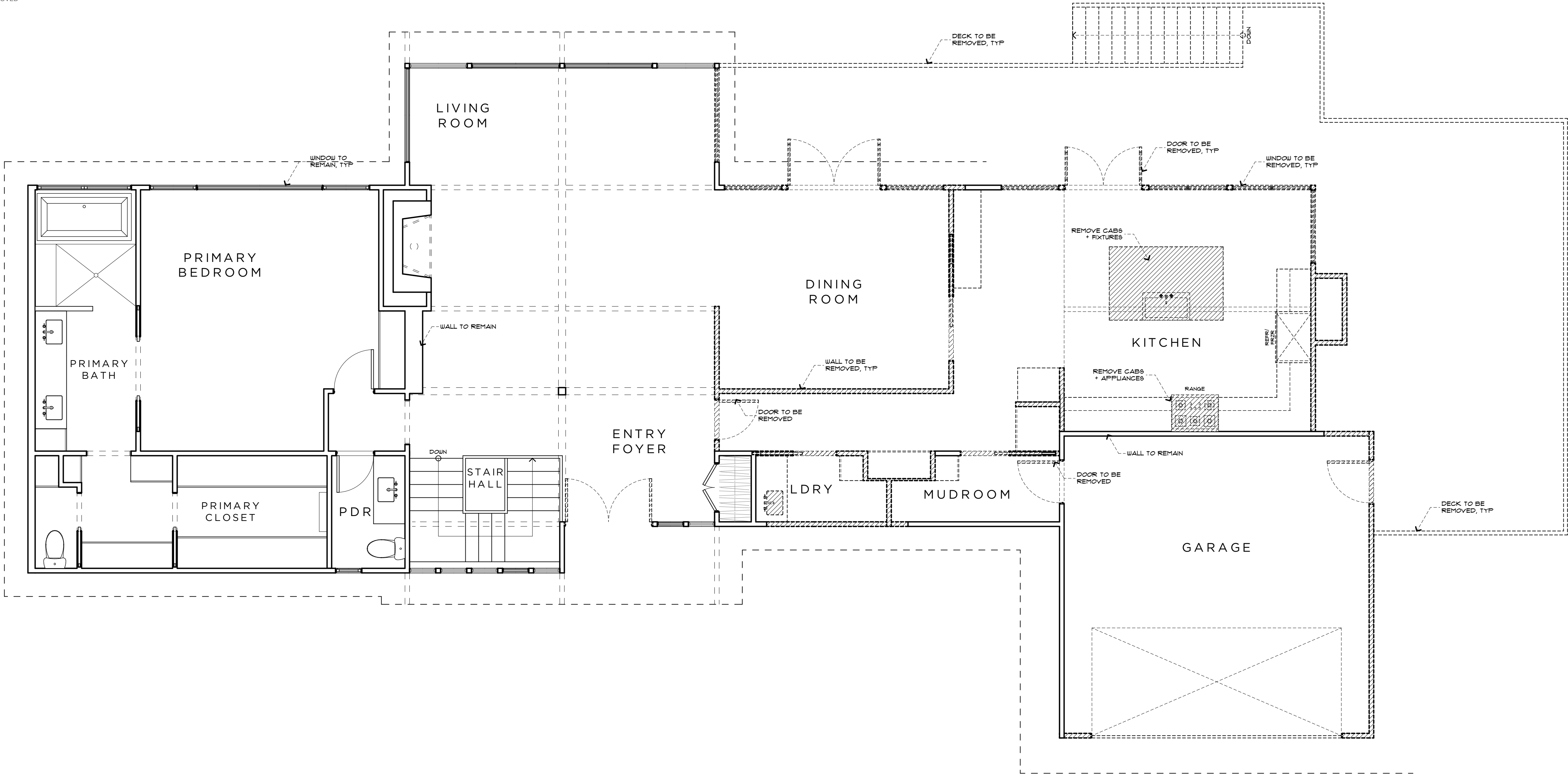
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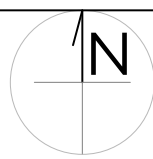
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FIRST FLOOR DEMOLITION PLAN

SCALE: 1/4" = 1'-0"





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FIRST FLOOR
DEMOLITION PLAN

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FLOOR AREAS:

LOT AREA:	16,678 FT ²
MAXIMUM ALLOWABLE GFA (R-15, 12,000 or 40%):	(40%) 6,671.20 FT ²
EX BASEMENT FLOOR AREA:	1,193 FT ²
EX BASEMENT 'GARAGE' AREA:	495 FT ²
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BASEMENT FLOOR PLAN

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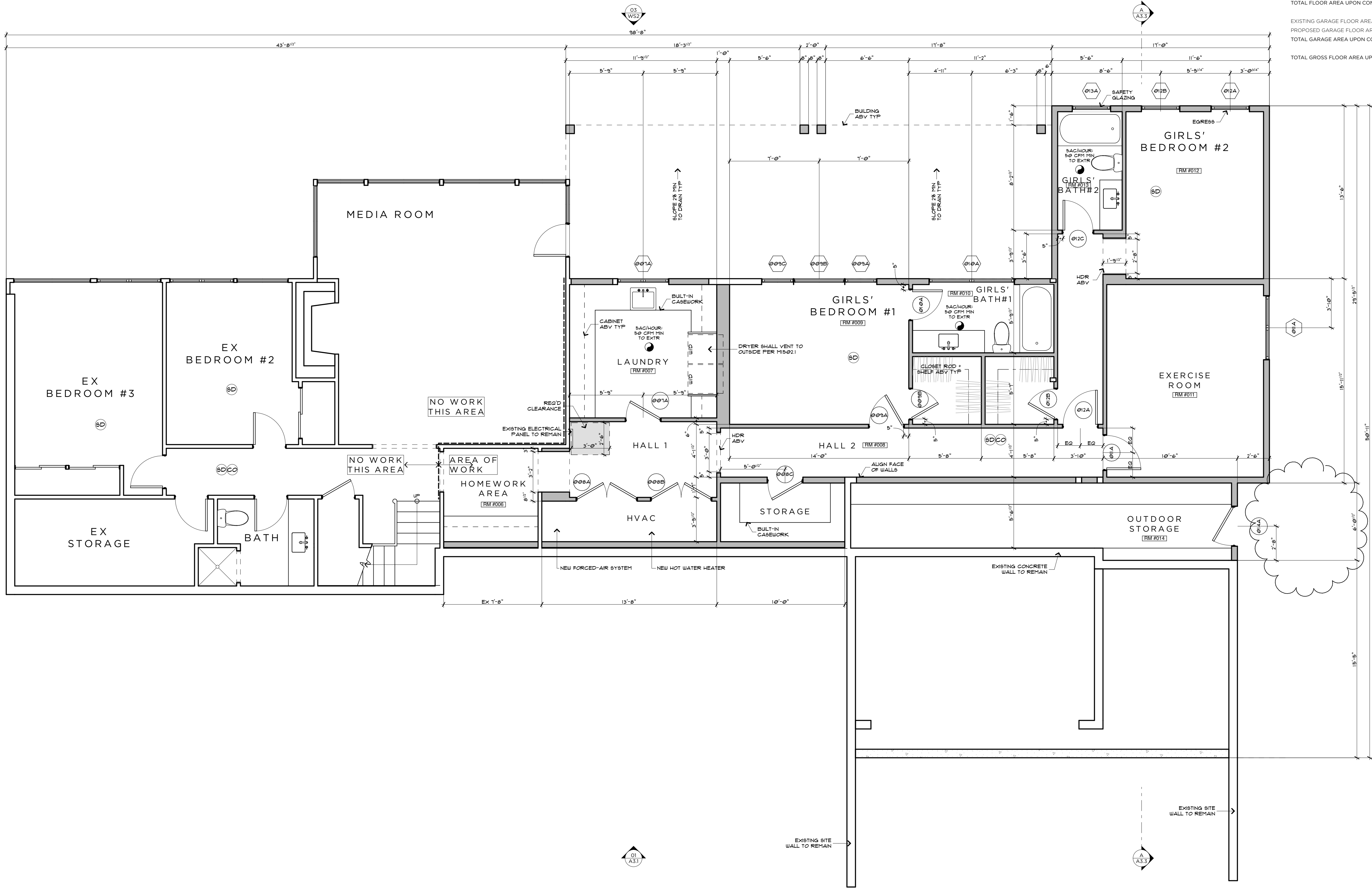
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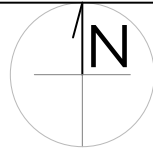
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BASEMENT PLAN

SCALE: 1/4" = 1'-0"



A 2 . 0

ALPAY
AUTHORED: 2/27/26

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PROPOSED GARAGE FLOOR AREA:	270 FT ²
TOTAL GARAGE AREA UPON COMPLETION:	654 FT ²
TOTAL GROSS FLOOR AREA UPON COMPLETION:	(35.2%) 5,879 FT ²



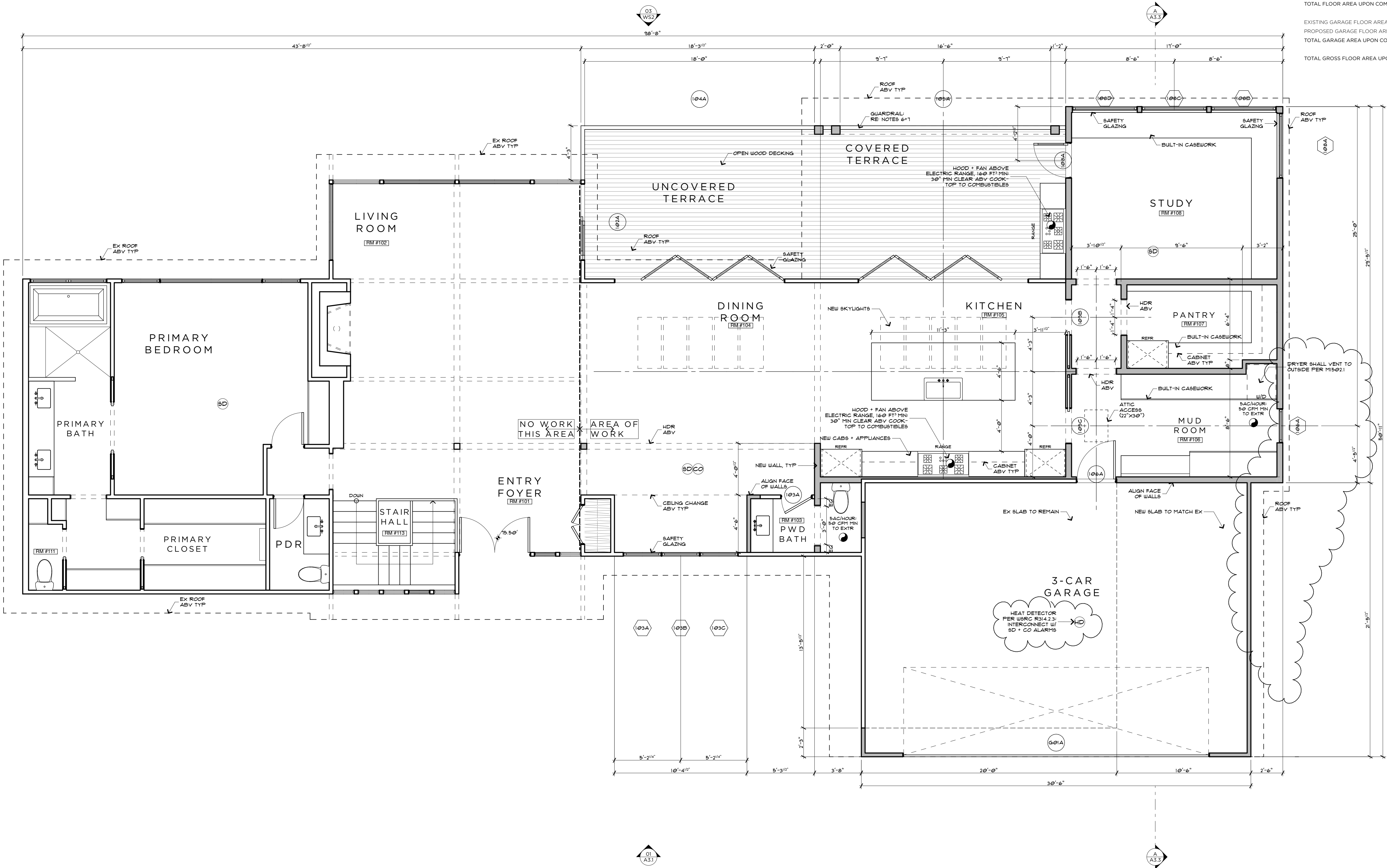
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206.913.2333

4303 STONE WAY N
SEATTLE, WA 98103

9790 REGISTERED ARCHITECT

JAMES M DEARTH
STATE OF WASHINGTON



FIRST FLOOR PLAN

SCALE: 1/4" = 1'-0"

ALPAY RESIDENCE
5419 96TH AVE SE MERCER ISLAND, WA 98040

FIRST FLOOR PLAN

RELEASE
PERMIT RESPONSE
9 DECEMBER 2025
CAR-2 PERMIT SUBMITTAL
23 DECEMBER 2025
PERMIT RESPONSE
7 JANUARY 2026
PERMIT RESPONSE
26 FEBRUARY 2026

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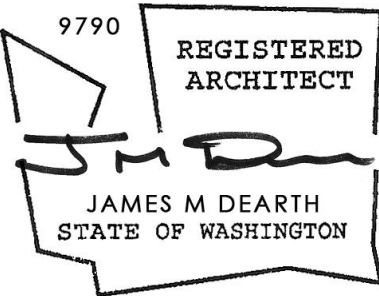
ALPAY
AUTHOR: 2/27/26



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ALPAY RESIDENCE
5419 96TH AVE SE MERCER ISLAND, WA 98040

ROOF PLAN

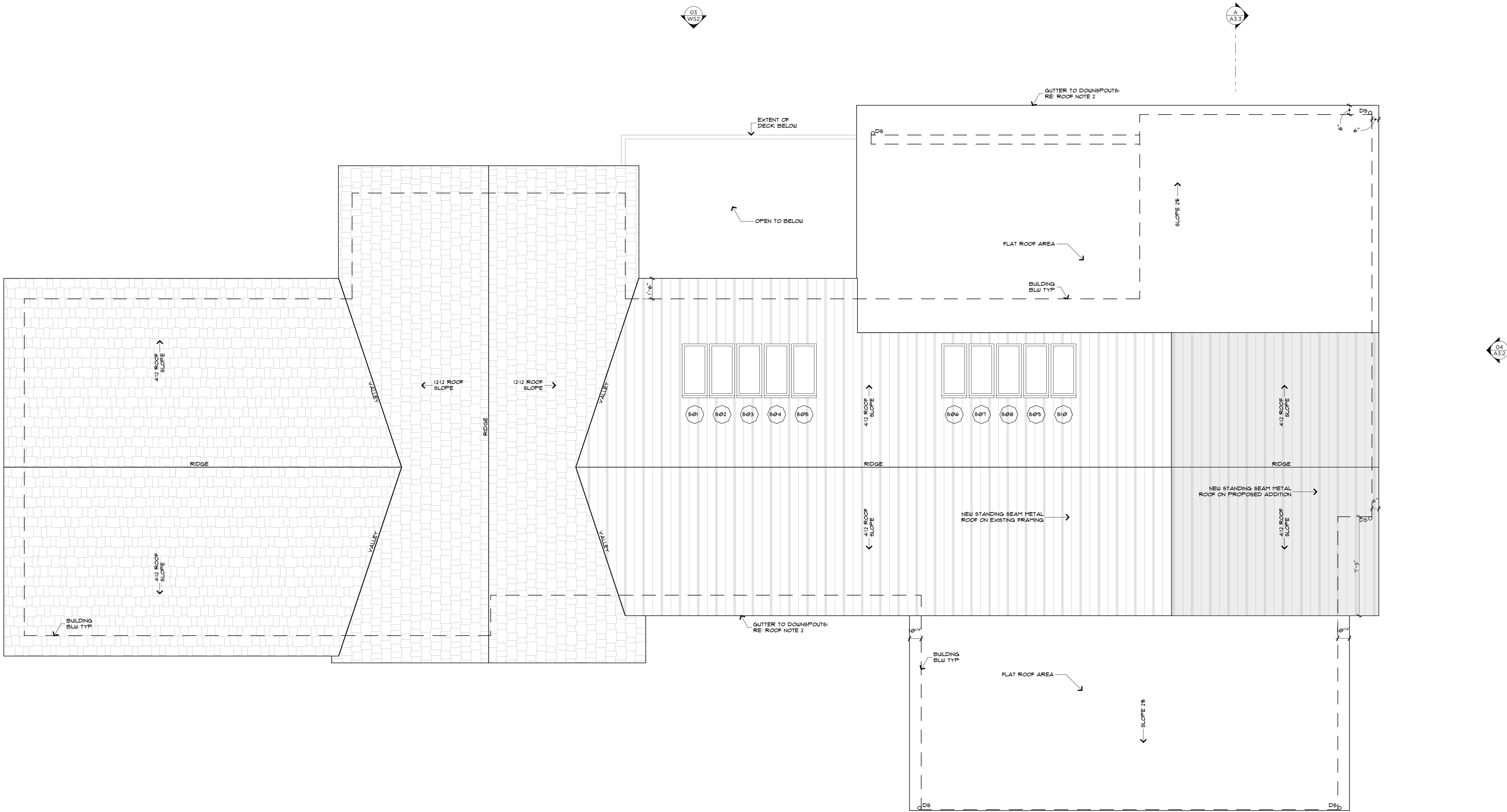
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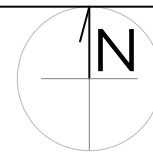
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ALPAY
AUTHORED: 2/27/26

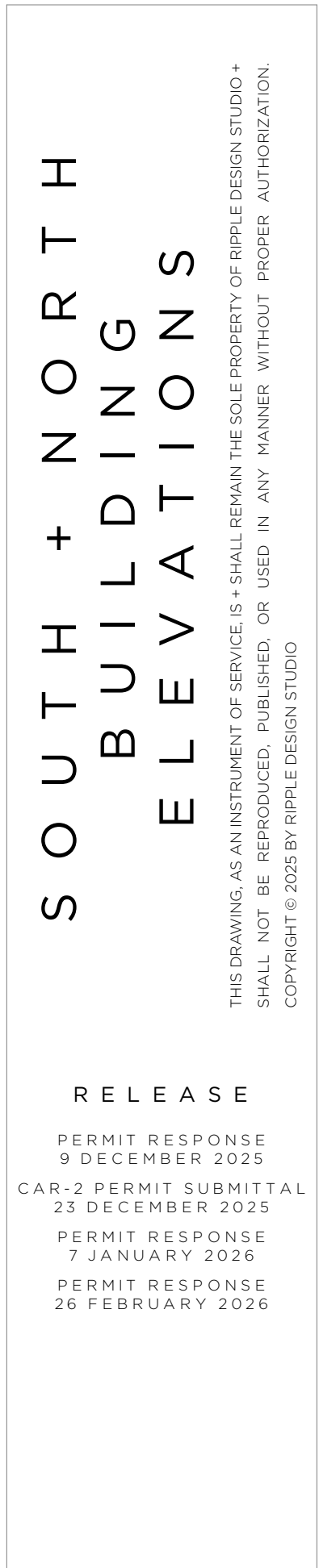
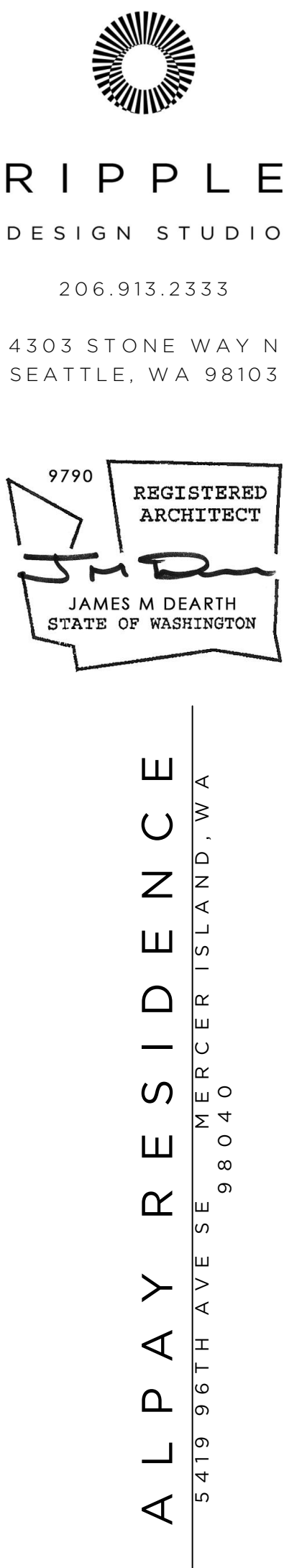


ROOF PLAN

SCALE: 1/4" = 1'-0"



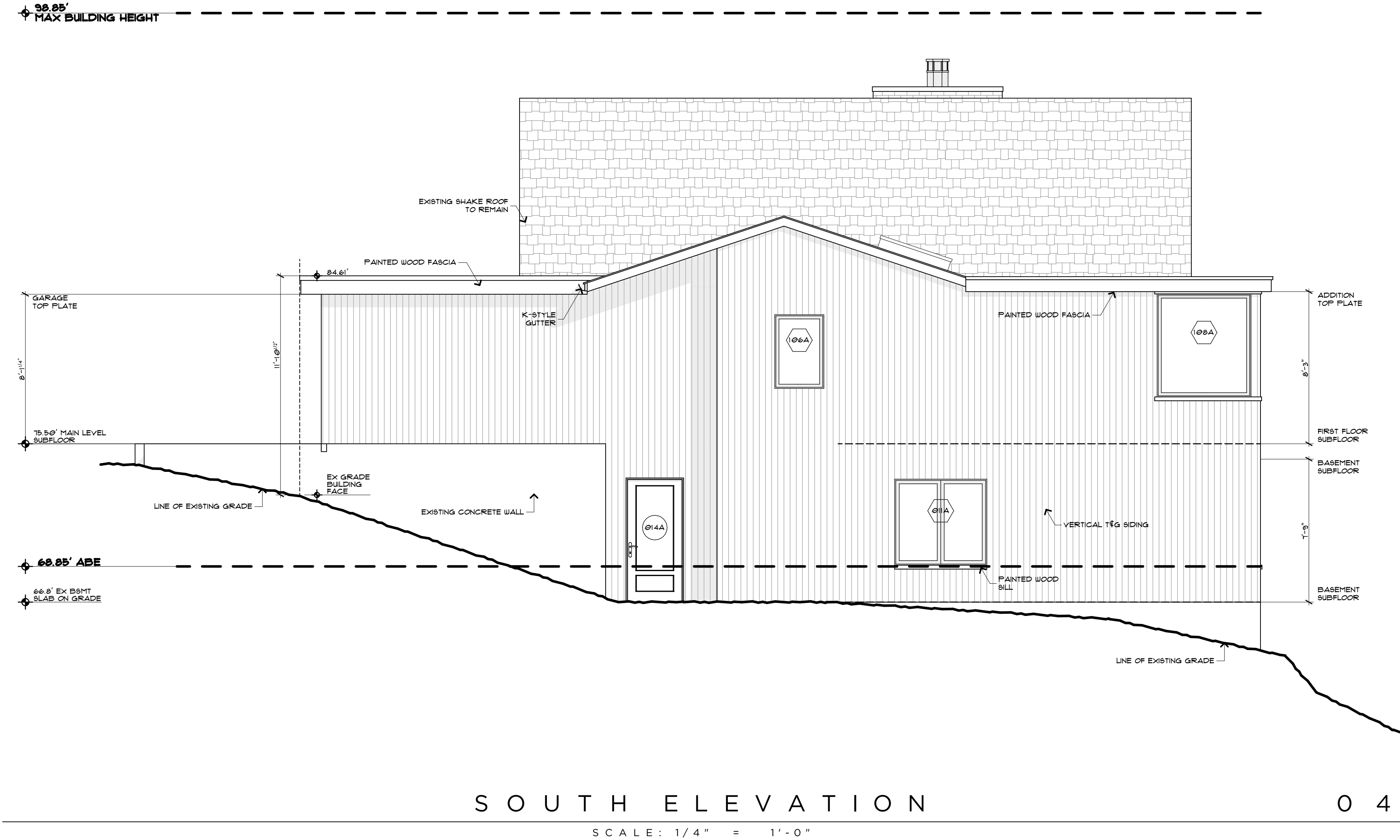
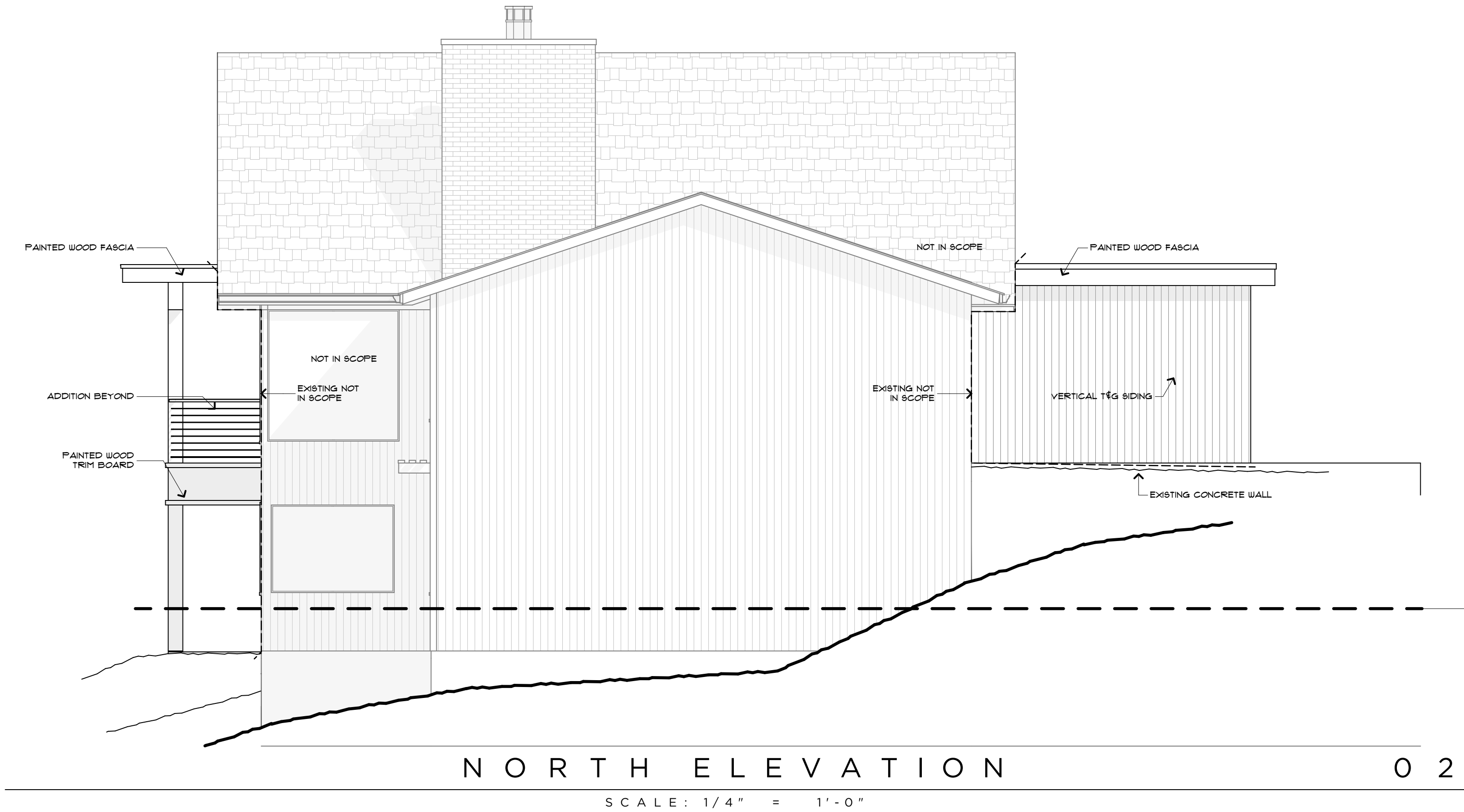
ELEVATION POINT "A"	75.30
SEGMENT LENGTH "A"	30.50
ELEV "A" x SEGMENT "A"	2,296.65
ELEVATION POINT "B"	73.00
SEGMENT LENGTH "B"	15.71
ELEV "B" x SEGMENT "B"	1,146.83
ELEVATION POINT "C"	74.00
SEGMENT LENGTH "C"	31.33
ELEV "C" x SEGMENT "C"	2,318.42
ELEVATION POINT "D"	75.45
SEGMENT LENGTH "D"	3.00
ELEV "D" x SEGMENT "D"	226.35
ELEVATION POINT "E"	73.60
SEGMENT LENGTH "E"	34.25
ELEV "E" x SEGMENT "E"	2,520.80
ELEVATION POINT "F"	66.10
SEGMENT LENGTH "F"	24.70
ELEV "F" x SEGMENT "F"	1,583.27
ELEVATION POINT "G"	65.20
SEGMENT LENGTH "G"	24.00
ELEV "G" x SEGMENT "G"	1,564.80
ELEVATION POINT "H"	65.50
SEGMENT LENGTH "H"	7.75
ELEV "H" x SEGMENT "H"	507.63
ELEVATION POINT "I"	66.00
SEGMENT LENGTH "I"	20.00
ELEV "I" x SEGMENT "I"	1,320.00
ELEVATION POINT "J"	66.70
SEGMENT LENGTH "J"	7.75
ELEV "J" x SEGMENT "J"	516.93
ELEVATION POINT "K"	66.60
SEGMENT LENGTH "K"	14.50
ELEV "K" x SEGMENT "K"	965.70
ELEVATION POINT "L"	66.30
SEGMENT LENGTH "L"	50.00
ELEV "L" x SEGMENT "L"	3,315.00
ELEVATION POINT "M"	64.50
SEGMENT LENGTH "M"	17.00
ELEV "M" x SEGMENT "M"	1,095.50
ELEVATION POINT "N"	66.00
SEGMENT LENGTH "N"	30.50
ELEV "N" x SEGMENT "N"	2,013.00
ELEVATION POINT "O"	66.90
SEGMENT LENGTH "O"	2.50
ELEV "O" x SEGMENT "O"	167.25
ELEVATION POINT "P"	70.00
SEGMENT LENGTH "P"	21.50
ELEV "P" x SEGMENT "P"	1,505.00
TOTAL OF ELEVATION POINTS x SEGMENT LENGTHS	23,064.12
TOTAL SEGMENT LENGTHS	354.99
AVERAGE GRADE	68.85



AVERAGE BUILDING ELEVATION CALCS:

ELEVATION POINT "A"	75.30
SEGMENT LENGTH "A"	30.50
ELEV "A" x SEGMENT "A"	2,296.65
ELEVATION POINT "B"	73.00
SEGMENT LENGTH "B"	15.71
ELEV "B" x SEGMENT "B"	1,146.83
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SEGMENT LENGTH "O"	2.50
ELEV "O" x SEGMENT "O"	167.25
ELEVATION POINT "P"	70.00
SEGMENT LENGTH "P"	21.50
ELEV "P" x SEGMENT "P"	1,505.00

TOTAL OF ELEVATION POINTS x SEGMENT LENGHTS	23,064.12
TOTAL SEGMENT LENGTHS	334.99
AVERAGE GRADE	68.85





RIPPLE
DESIGN STUDIO

206.913.2333

4303 STONE WAY N
SEATTLE, WA 98103

9790
REGISTERED
ARCHITECT
JAMES M DEARTH
STATE OF WASHINGTON

ALPAY RESIDENCE
5419 96TH AVE SE
MERCER ISLAND, WA
98040

NORTH + WEST
BUILDING
ELEVATIONS

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SECTION +
ELEVATION NOTES:

1. CHIMNEY SHALL EXTEND A MIN OF 2'-0" ABV ROOF OR PARAPET WITHIN 10'-0" RADIUS OF CHIMNEY. PROVIDE APPROVED SPARK ARRESTOR @ ALL CHIMNEY CAPS. ALL ARCHITECTURAL FEATURES MUST BE PERMITTED BY FLU + SPARK ARRESTOR MFR APPROVAL.
2. OPEN GUARDRAILS SHALL HAVE INTERMEDIATE RAILS OR ORNAMENTAL PATTERN SUCH THAT A 4" SPHERE CANNOT PASS THROUGH.

WINDOW SCHEDULE:

ALL SCHEDULED DIMENSIONS ARE NOMINAL. RE: MFR DOCUMENTATION + HEAD HEIGHT DETAIL, THIS SHEET, FOR ACTUAL DIMENSIONS

WDW #	WIDTH	HEIGHT	HEADER	TYPE	FINISH	GLAZING	WDW COVERING	OPERATION	HARDWARE	NOTES
007A	4'-0"	3'-8"	6'-8"	CASEMENT	PAINTED	NONE	---	---	TBD	
009A	2'-6"	4'-8"	6'-8"	CASEMENT	PAINTED	NONE	---	---	TBD	EGRESS
009B	4'-0"	4'-8"	6'-8"	FIXED	PAINTED	NONE	---	---	TBD	
009C	2'-6"	4'-8"	6'-8"	CASEMENT	PAINTED	NONE	---	---	TBD	EGRESS
010A	6'-8"	3'-8"	6'-8"	CASEMENT	PAINTED	NONE	---	---	TBD	
011A	5'-0"	4'-8"	6'-8"	CASEMENT	PAINTED	NONE	---	---	TBD	
012A	3'-0"	4'-8"	6'-8"	CASEMENT	PAINTED	NONE	---	---	TBD	EGRESS
012B	3'-0"	4'-8"	6'-8"	CASEMENT	PAINTED	NONE	---	---	TBD	EGRESS
013A	3'-0"	4'-8"	6'-8"	FIXED	PAINTED	NONE	---	---	TBD	
103A	3'-0"	7'-0"	7'-0"	FIXED	PAINTED	NONE	---	---	TBD	
103B	3'-0"	7'-0"	7'-0"	FIXED	PAINTED	NONE	---	---	TBD	
103C	3'-0"	7'-0"	7'-0"	FIXED	PAINTED	NONE	---	---	TBD	
106A	2'-8"	4'-0"	7'-0"	FIXED	PAINTED	NONE	---	---	TBD	
108A	5'-1"	5'-7"	8'-1 1/2"	FIXED	PAINTED	NONE	---	---	TBD	
108B	5'-1"	5'-7"	8'-1 1/2"	FIXED	PAINTED	NONE	---	---	TBD	
108C	5'-1"	5'-7"	8'-1 1/2"	FIXED	PAINTED	NONE	---	---	TBD	
108D	5'-1"	5'-7"	8'-1 1/2"	FIXED	PAINTED	NONE	---	---	TBD	

EXTERIOR DOOR SCHEDULE:

ALL SCHEDULED DIMENSIONS ARE NOMINAL. RE: MFR DOCUMENTATION + HEAD HEIGHT DETAIL, THIS SHEET, FOR ACTUAL DIMENSIONS

DOOR #	WIDTH	HEIGHT	TYPE	DOOR LEAF	MATERIAL	FINISH	GLAZING	HARDWARE	NOTES
014A	3'-0"	6'-8"	SWING	PANEL	WOOD	PAINTED	NONE	TBD	
102A	6'-0"	7'-0"	SLIDER	FULL LIGHT	CLAD WOOD	PAINTED	L6E3 366	TBD	
104A	13'-4"	7'-0"	MULTI-SLIDE	FULL LIGHT	CLAD WOOD	PAINTED	L6E3 366	TBD	
105A	13'-4"	7'-0"	MULTI-SLIDE	FULL LIGHT	CLAD WOOD	PAINTED	L6E3 366	TBD	
108A	2'-8"	7'-0"	SWING	PANEL	WOOD	PAINTED	NONE	TBD	
G01A	24'-0"	7'-0"	OVERHEAD	PANEL	STEEL	PAINTED	NONE	TBD	

SKYLIGHT SCHEDULE:

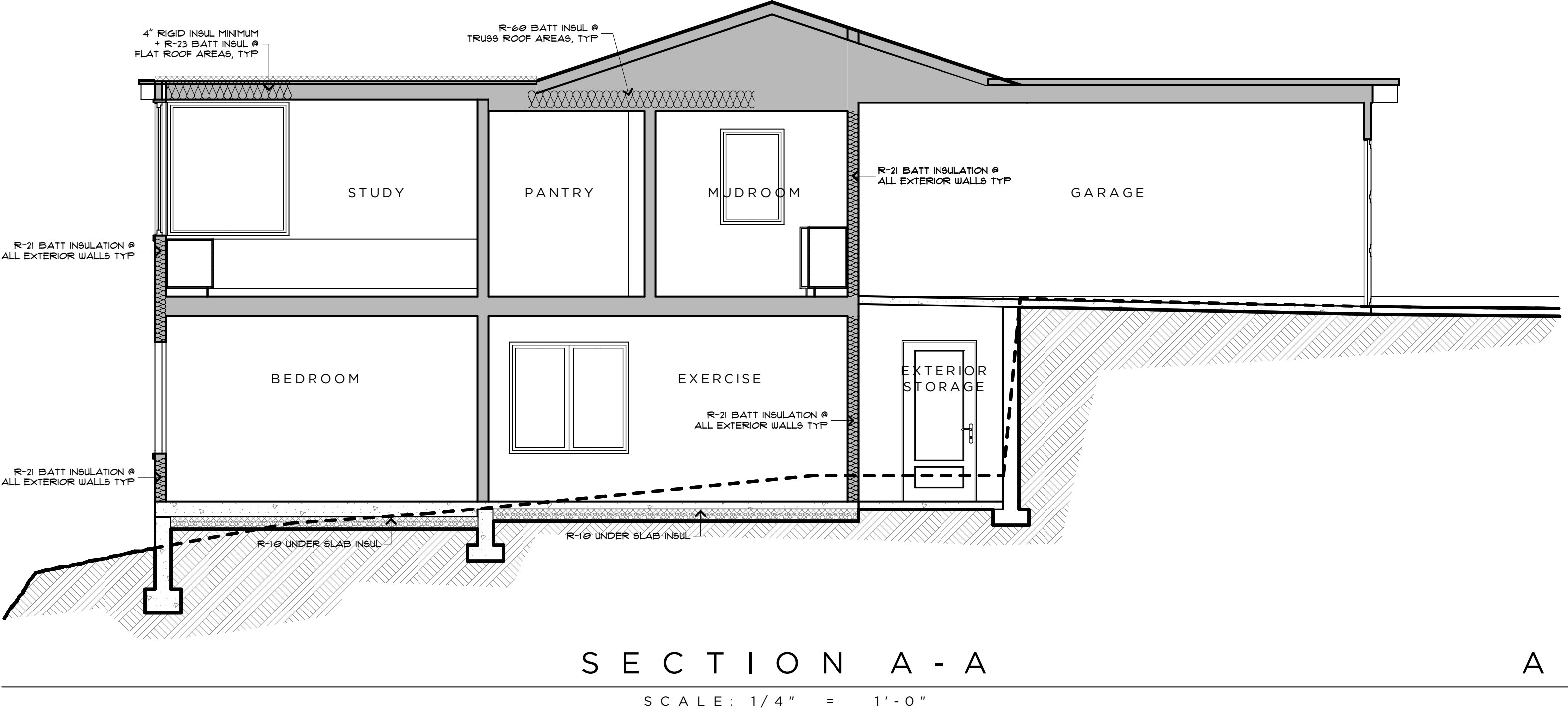
ALL SCHEDULED DIMENSIONS ARE NOMINAL. RE: MFR DOCUMENTATION + HEAD HEIGHT DETAIL, THIS SHEET, FOR ACTUAL DIMENSIONS

WDW #	WIDTH	HEIGHT	TYPE	FINISH	GLAZING	HARDWARE	NOTES
S01	1'-10"	4'-0"	FIXED	PAINTED	L6E3 366	TBD	
S02	1'-10"	4'-0"	FIXED	PAINTED	L6E3 366	TBD	
S03	1'-10"	4'-0"	FIXED	PAINTED	L6E3 366	TBD	
S04	1'-10"	4'-0"	FIXED	PAINTED	L6E3 366	TBD	
S05	1'-10"	4'-0"	FIXED	PAINTED	L6E3 366	TBD	
S06	1'-10"	4'-0"	FIXED	PAINTED	L6E3 366	TBD	
S07	1'-10"	4'-0"	FIXED	PAINTED	L6E3 366	TBD	
S08	1'-10"	4'-0"	FIXED	PAINTED	L6E3 366	TBD	
S09	1'-10"	4'-0"	FIXED	PAINTED	L6E3 366	TBD	
S10	1'-10"	4'-0"	FIXED	PAINTED	L6E3 366	TBD	

INTERIOR DOOR SCHEDULE:

ALL SCHEDULED DIMENSIONS ARE NOMINAL. RE: HEAD HEIGHT DETAIL, THIS SHEET, FOR ACTUAL DIMENSIONS

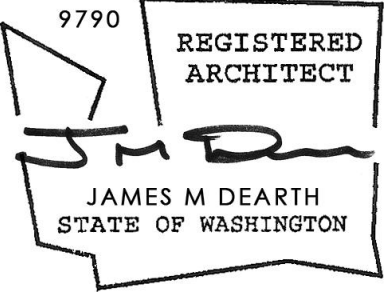
DOOR #	WIDTH	HEIGHT	TYPE	DOOR LEAF	MATERIAL	FINISH	HARDWARE	LATCHING	NOTES
007A	2'-8"	6'-8"	SWING	PANEL	WOOD	PAINTED	TBD	PASSAGE	
008A	4'-8"	7'-0"	SWING	PANEL	WOOD	PAINTED	TBD	PASSAGE	
008B	4'-8"	7'-0"	SWING	PANEL	WOOD	PAINTED	TBD	PASSAGE	
008C	2'-8"	6'-8"	SWING	PANEL	WOOD	PAINTED	TBD	PASSAGE	
009A	2'-8"	6'-8"	SWING	PANEL	WOOD	PAINTED	TBD	PASSAGE	
009B	2'-4"	6'-8"	SWING	PANEL	WOOD	PAINTED	TBD	PASSAGE	
010A	2'-4"	6'-8"	SWING	PANEL	WOOD	PAINTED	TBD	PASSAGE	
011A	2'-8"	6'-8"	SWING	PANEL	WOOD	PAINTED	TBD	PASSAGE	
012A	2'-8"	6'-8"	SWING	PANEL	WOOD	PAINTED	TBD	PASSAGE	
012B	2'-4"	6'-8"	SWING	PANEL	WOOD	PAINTED	TBD	PASSAGE	
012C	2'-4"	6'-8"	SWING	PANEL	WOOD	PAINTED	TBD	PASSAGE	
103A	2'-4"	6'-8"	SWING	PANEL	WOOD	PAINTED	TBD	PASSAGE	
105B	2'-8"	8'-0"	SWING	PANEL	WOOD	PAINTED	TBD	PASSAGE	
105C	2'-8"	8'-0"	SWING	PANEL	WOOD	PAINTED	TBD	PASSAGE	
106A	3'-0"	6'-8"	SWING	PANEL	WOOD	PAINTED	TBD	PASSAGE	



RIPPLE
DESIGN STUDIO

206.913.2333

4303 STONE WAY N
SEATTLE, WA 98103



ALPAY RESIDENCE
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BUILDING
SECTIONS A - A + B -

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Criteria

1. ALL MATERIALS, WORKMANSHIP, DESIGN, AND CONSTRUCTION SHALL CONFORM TO THE DRAWINGS, SPECIFICATIONS, AND THE 2021 INTERNATIONAL BUILDING CODE.

2. DESIGN LOAD CRITERIA
- | | |
|--|---|
| FLOOR LIVE LOAD (PARKING) | 40 PSF |
| FLOOR CONCENTRATED LOAD (PARKING) | 3000 LBS |
| FLOOR LIVE LOAD (RESIDENTIAL) | 40 PSF |
| FLOOR LIVE LOAD (RESIDENTIAL DECKS) | 60 PSF |
| GUARDRAILS/BALCONY RAILS CONCENTRATED LOAD | 200 LBS |
| SNOW | Pf=25 PSF |
| WIND | Iw=1.0, GCpi=0.18, 98 MPH (ULTIMATE), EXPOSURE "C", KZT=1.0 |

EARTHQUAKE

ANALYSIS PROCEDURE:

LATERAL SYSTEM:

BASE SHEAR (ALLOWABLE)

SITE CRITERIA

SEE PLANS FOR ADDITIONAL LOADING CRITERIA

3. STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH ARCHITECTURAL DRAWINGS FOR BIDDING AND CONSTRUCTION. CONTRACTOR SHALL VERIFY DIMENSIONS AND CONDITIONS FOR COMPATIBILITY AND SHALL NOTIFY ARCHITECT OF ANY DISCREPANCIES PRIOR TO CONSTRUCTION.

4. CONTRACTOR SHALL VERIFY ALL EXISTING DIMENSIONS, MEMBER SIZES, AND CONDITIONS PRIOR TO COMMENCING ANY WORK. ALL DIMENSIONS OF EXISTING CONSTRUCTION SHOWN ON THE DRAWINGS ARE INTENDED AS GUIDELINES ONLY AND MUST BE VERIFIED.

5. CONTRACTOR SHALL PROVIDE TEMPORARY BRACING FOR THE STRUCTURE AND STRUCTURAL COMPONENTS UNTIL ALL FINAL CONNECTIONS HAVE BEEN COMPLETED IN ACCORDANCE WITH THE PLANS.

6. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SAFETY PRECAUTIONS AND THE METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES, REQUIRED TO PERFORM THE CONTRACTORS WORK. THE STRUCTURAL ENGINEER HAS NO OVERALL SUPERVISORY AUTHORITY OR ACTUAL AND/OR DIRECT RESPONSIBILITY FOR THE SPECIFIC WORKING CONDITIONS AT THE SITE AND/OR FOR ANY HAZARDS RESULTING FROM THE ACTIONS OF ANY TRADE CONTRACTOR. THE STRUCTURAL ENGINEER HAS NO DUTY TO INSPECT, SUPERVISE, NOTE, CORRECT, OR REPORT ANY HEALTH OR SAFETY DEFICIENCIES TO THE OWNER, CONTRACTORS, OR OTHER ENTITIES OR PERSONS AT THE PROJECT SITE.

7. CONTRACTOR-INITIATED CHANGES SHALL BE SUBMITTED IN WRITING TO THE ARCHITECT AND STRUCTURAL ENGINEER FOR APPROVAL PRIOR TO FABRICATION OR CONSTRUCTION. CHANGES SHOWN ON SHOP DRAWINGS ONLY WILL NOT SATISFY THIS REQUIREMENT.

8. DRAWINGS INDICATE GENERAL AND TYPICAL DETAILS OF CONSTRUCTION. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, SIMILAR DETAILS OF CONSTRUCTION SHALL BE USED, SUBJECT TO REVIEW AND APPROVAL BY THE ARCHITECT AND THE STRUCTURAL ENGINEER.

9. ALL STRUCTURAL SYSTEMS WHICH ARE TO BE COMPOSED OF COMPONENTS TO BE FIELD ERECTED SHALL BE SUPERVISED BY THE SUPPLIER DURING MANUFACTURING, DELIVERY, HANDLING, STORAGE AND ERECTION IN ACCORDANCE WITH INSTRUCTIONS PREPARED BY THE SUPPLIER.

10. SHOP DRAWINGS FOR THE FOLLOWING ITEMS SHALL BE SUBMITTED TO THE ARCHITECT AND STRUCTURAL ENGINEER FOR REVIEW PRIOR TO FABRICATION OF THESE ITEMS.

CONNECTOR PLATE WOOD ROOF TRUSSES

STRUCTURAL STEEL

CONTRACTOR SHALL SUBMIT WALL ELEVATION DRAWINGS OF AT LEAST 1/8"=1'-0" SCALE INDICATING LOCATIONS OF CONNECTION EMBEDMENTS AND WALL OPENINGS FOR REVIEW PRIOR TO CONSTRUCTION. CONTRACTOR SHALL COORDINATE WITH REINFORCEMENT SHOP DRAWINGS.

APPROVED SETS OF ALL SHOP DRAWINGS SHALL ALSO BE SUBMITTED TO THE BUILDING DEPARTMENT.

11. SHOP DRAWING REVIEW: DIMENSIONS AND QUANTITIES ARE NOT REVIEWED BY THE ENGINEER OF RECORD, THEREFORE MUST BE VERIFIED BY THE CONTRACTOR. CONTRACTOR SHALL REVIEW AND STAMP DRAWINGS PRIOR TO REVIEW BY ENGINEER OF RECORD. CONTRACTOR SHALL REVIEW DRAWINGS FOR CONFORMANCE WITH THE MEANS, METHODS, TECHNIQUES, SEQUENCES AND OPERATIONS OF CONSTRUCTION, AND ALL SAFETY PRECAUTIONS AND PROGRAMS INCIDENTAL THERETO. SUBMITTALS SHALL INCLUDE A REPRODUCIBLE AND ONE COPY; REPRODUCIBLE WILL BE MARKED AND RETURNED WITHIN TWO WEEKS OF RECEIPT WITH A NOTATION INDICATING THAT THE SUBMITTAL HAS BEEN FOUND TO BE IN GENERAL CONFORMANCE WITH THE DESIGN OF THE BUILDING. THE SUBMITTED ITEMS SHALL NOT BE INSTALLED UNTIL THEY HAVE BEEN APPROVED BY THE BUILDING OFFICIAL.

SHOP DRAWING SUBMITTALS PROCESSED BY THE ENGINEER ARE NOT CHANGE ORDERS. THE PURPOSE OF SHOP DRAWING SUBMITTALS BY THE CONTRACTOR IS TO DEMONSTRATE TO THE ENGINEER THAT THE CONTRACTOR UNDERSTANDS THE DESIGN CONCEPT, BY INDICATING WHICH MATERIAL IS INTENDED TO BE FURNISHED AND INSTALLED AND BY DETAILING THE INTENDED FABRICATION AND INSTALLATION METHODS. IF DEVIATIONS, DISCREPANCIES, OR CONFLICTS BETWEEN SHOP DRAWING SUBMITTALS AND THE CONTRACT DOCUMENTS ARE DISCOVERED EITHER PRIOR TO OR AFTER SHOP DRAWING SUBMITTALS ARE PROCESSED BY THE ENGINEER, THE DESIGN DRAWINGS AND SPECIFICATIONS SHALL CONTROL AND SHALL BE FOLLOWED.

Quality Assurance

1. SPECIAL INSPECTION SHALL BE PROVIDED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS AND SECTIONS 110 AND 1704 OF THE INTERNATIONAL BUILDING CODE BY A QUALIFIED TESTING AGENCY DESIGNATED BY THE ARCHITECT, AND RETAINED BY THE BUILDING OWNER, THE ARCHITECT, STRUCTURAL ENGINEER, AND BUILDING DEPARTMENT SHALL BE FURNISHED WITH COPIES OF ALL INSPECTION AND TEST RESULTS. SPECIAL INSPECTION IS REQUIRED OF THE FOLLOWING TYPES OF CONSTRUCTION:

STRUCTURAL STEEL FABRICATION AND ERECTION

PER IBC 1705.2

PER IBC TABLE 1705.7

PER SOILS REPORT

EXPANSION BOLTS AND THREADED EXPANSION INSERTS

PER MANUFACTURER

EPOXY GROUTED INSTALLATIONS

PER MANUFACTURER

Geotechnical

1. FOUNDATION NOTES: SUBGRADE PREPARATION INCLUDING DRAINAGE, EXCAVATION, COMPACTION, AND FILLING REQUIREMENTS, SHALL CONFORM STRICTLY WITH RECOMMENDATIONS GIVEN IN THE SOILS REPORT OR AS DIRECTED BY THE SOILS ENGINEER. FOOTINGS SHALL BEAR ON SOLID UNDISTURBED EARTH AT LEAST 18" BELOW LOWEST ADJACENT FINISHED GRADE. FOOTING DEPTHS/ELEVATIONS SHOWN ONPLANS (OR IN DETAILS) ARE MINIMUM AND FOR GUIDANCE ONLY; THE ACTUAL ELEVATIONS OF FOOTINGS MUST BE ESTABLISHED BY THE CONTRACTOR IN THE FIELD WORKING WITH THE TESTING LAB AND SOILS ENGINEER. BACKFILL BEHIND ALL RETAINING WALLS WITH FREE DRAINING GRANULAR FILL AND PROVIDE FOR SUBSURFACE DRAINAGE AS NOTED IN THE SOILS REPORT.

ALLOWABLE SOIL PRESSURE

2000 PSF

LATERAL EARTH PRESSURE (UNRESTRAINED)

40 PCF

PASSIVE EARTH PRESSURE (AT H PILES)

350 PCF

3" PILE CAPACITY (COMPRESSION)

12 Kips

DRIVEN H PILE

20 Kips

SOILS REPORT REFERENCE: Geotech Consultants, Inc, ProjectJN 25288, Date: 10/24/2025

2. AUGERCAST PILING INSPECTION BY THE SOILS ENGINEER SHALL BE PERFORMED DURING PLACEMENT. MAXIMUM AUGERCAST PILE ECCENTRICITY SHALL BE 3" Laterally.PILE LENGTH INDICATED ON DRAWINGS IS ESTIMATED. ACTUAL LENGTH SHALL BE DETERMINED IN FIELD BY SOILS ENGINEER. THE CONTRACTOR SHALL DETERMINE THE LOCATION OF ALL ADJACENT UNDERGROUND UTILITIES PRIOR TO DRILLING PILES.

3. 3" PIN PILES SHALL BE DRIVEN TO REFUSAL AS DEFINED BY THE GEOTECHNICAL REPORT. LOAD TESTING SHALL BE CONDUCTED UNDER THE SUPERVISION OF THE GEOTECHNICAL ENGINEER USING THE ASTM D 1143-81 QUICK TEST METHOD. PERFORM A 200% LOAD TEST ON 3% OF PILES (1 MINIMUM, 5 MAXIMUM).
- Renovation
1. DEMOLITION: CONTRACTOR SHALL VERIFY ALL EXISTING CONDITIONS BEFORE COMMENCING ANY DEMOLITION. SHORING SHALL BE INSTALLED TO SUPPORT EXISTING CONSTRUCTION AS REQUIRED AND IN A MANNER SUITABLE TO THE WORK SEQUENCES. DEMOLITION DEBRIS SHALL NOT BE ALLOWED TO DAMAGE OR OVERLOAD THE EXISTING STRUCTURE. LIMIT CONSTRUCTION LOADING (INCLUDING DEMOLITION DEBRIS) ON EXISTING FLOOR SYSTEMS TO 40 PSF.

2. CONTRACTOR SHALL CHECK FOR DRYROT AT ALL AREAS OF NEW WORK. ALL ROT SHALL BE REMOVED AND DAMAGED MEMBERS SHALL BE REPLACED OR REPAIRED AS DIRECTED BY THE STRUCTURAL ENGINEER OR ARCHITECT.
- Concrete
1. CONCRETE SHALL BE MIXED, PROPORTIONED, CONVEYED AND PLACED IN ACCORDANCE WITH IBC SECTION 1905, 1906, AND ACI 301. STRENGTHS AT 28 DAYS AND MIX CRITERIA SHALL BE AS FOLLOWS:
- | TYPE OF CONSTRUCTION | 28 DAY STRENGTH (F'c) |
|--|-----------------------|
| ALL STRUCTURAL CONCRETE | 3,000 PSI |
| NOTE: STRUCTURAL DESIGN OF FOUNDATION IS BASED ON A F'c=2,500 PSI, PER IBC 1705.3.2.3, SPECIAL INSPECTION IS NOT REQUIRED | |
| 2. ALL CONCRETE WITH SURFACES EXPOSED TO WEATHER OR STANDING WATER SHALL BE AIR-ENTRAINED WITH AN AIR-ENTRAINING AGENT CONFORMING TO ASTM C260, C494, AND C618. TOTAL AIR CONTENT FOR FROST-RESISTANT CONCRETE SHALL BE IN ACCORDANCE WITH TABLE ACI 318 TABLE 19.3.1.1 MODERATE EXPOSURE. | |
| 3. REINFORCING STEEL SHALL CONFORM TO ASTM A615 (INCLUDING SUPPLEMENT S1), GRADE 60, fy=60,000 PSI. EXCEPTIONS: ANY BARS SPECIFICALLY SO NOTED ON THE DRAWINGS SHALL BE GRADE 40, fy=40,000 PSI. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185. | |
| 4. DETAILING OF REINFORCING STEEL (INCLUDING HOOKS AND BENDS) SHALL BE IN ACCORDANCE WITH ACI 315-99 AND 318-14. LAP ALL REINFORCEMENTS IN ACCORDANCE WITH "THE REINFORCING SPLICE AND DEVELOPMENT LENGTH SCHEDULE." PROVIDE CORNER BARS AT ALL WALL AND FOOTING INTERSECTIONS. LAP ADJACENT MATS OF WELDED WIRE FABRIC A MINIMUM OF 8" AT SIDES AND ENDS. NO BARS PARTIALLY EMBEDDED IN HARDENED CONCRETE SHALL BE FIELD BENT UNLESS SPECIFICALLY SO DETAILED OR APPROVED BY THE STRUCTURAL ENGINEER. | |
| 5. CONCRETE PROTECTION (COVER) FOR REINFORCING STEEL SHALL BE AS FOLLOWS: <div><div>FOOTINGS AND OTHER UNFORMED SURFACES CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH FORMED SURFACES EXPOSED TO EARTH OR WEATHER (#5 BARS OR SMALLER)</div><div>1-1/2"</div><div>SLABS AND WALLS (INT. FACE)</div><div>GREATER OF BAR DIAMETER PLUS 1/8" OR 3/4"</div></div> | |
| 6. NON-SHRINK GROUT SHALL BE FURNISHED BY AN APPROVED MANUFACTURER AND SHALL BE MIXED AND PLACED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S PUBLISHED RECOMMENDATIONS. GROUT STRENGTH SHALL BE AT LEAST EQUAL TO THE MATERIAL ON WHICH IT IS PLACED (3000 PSI MINIMUM). | |
- The Following Apply Unless Noted Otherwise on the Drawings
- Anchorage
1. WHERE EXPANSION BOLTS ARE USED, EITHER OF THE FOLLOWING PRODUCTS ARE ACCEPTABLE.

1.1. EXPANSION BOLTS INTO CONCRETE AND CONCRETE MASONRY UNITS SHALL BE "KWIK BOLT TZ" AS MANUFACTURED BY THE HILTI CORP., INSTALLED IN STRICT ACCORDANCE WITH ICC-ES REPORT NO. ESR-1917, INCLUDING MINIMUM EMBEDMENT REQUIREMENTS. BOLTS INTO CONCRETE MASONRY OR BRICK MASONRY UNITS SHALL BE INTO FULLY GROUTED CELLS. SUBSTITUTES PROPOSED BY CONTRACTOR SHALL BE SUBMITTED FOR REVIEW WITH ICC REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION IS REQUIRED FOR ALL EXPANSION BOLT INSTALLATION.

1.2. EXPANSION BOLTS INTO CONCRETE AND CONCRETE MASONRY UNITS SHALL BE STRONG-BOLT 2 ANCHORS AS MANUFACTURED BY THE SIMPSON STRONG TIE COMPANY AND INSTALLED IN STRICT CONFORMANCE TO ICC-ES REPORT NUMBER ESR-3037, INCLUDING MINIMUM EMBEDMENT REQUIREMENTS. BOLTS INTO CONCRETE MASONRY OR BRICK MASONRY UNITS SHALL BE INTO FULLY GROUTED CELLS. SPECIAL INSPECTION IS REQUIRED FOR ALL EXPANSION BOLT INSTALLATION.

2. WHERE EPOXY-GROUTED ITEMS ARE USED, EITHER OF THE FOLLOWING PRODUCTS ARE ACCEPTALBE.

2.1. EPOXY-GROUTED ITEMS (THREADED RODS OR REINFORCING BAR) SPECIFIED ON THE DRAWINGS SHALL BE INSTALLED USING "HIT RE 500-V3" AS MANUFACTURED BY HILTI CORP. INSTALL IN STRICT ACCORDANCE WITH ICC-ES REPORT NO. ESR-2322. SPECIAL INSPECTION OF INSTALLATION IS REQUIRED. RODS SHALL BE ASTM A-36 UNLESS OTHERWISE NOTED.

2.2. EPOXY-GROUTED ITEMS (THREADED RODS OR REINFORCING BAR) SPECIFIED ON THE DRAWINGS SHALL BE INSTALLED USING "SET-3G" HIGH-STRENGTH EPOXY AS MANUFACTURED BY THE SIMPSON STRONG TIE COMPANY. INSTALL IN STRICT ACCORDANCE WITH ICC-ES REPORT NO. ESR-4057. SPECIAL INSPECTION OF INSTALLATION IS REQUIRED. RODS SHALL BE ASTM A-36 UNLESS OTHERWISE NOTED.
- Steel
1. STRUCTURAL STEEL DESIGN, FABRICATION, AND ERECTION SHALL BE BASED ON:

A. EITHER AISC 360 AND SECTION 2205.2 OF THE INTERNATIONAL BUILDING CODE.

B. JULY 7, 2016 AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS, AMENDED AS FOLLOWS.

i) AS NOTED IN THE CONTRACT DOCUMENTS.

ii) BY THE DELETION OF PARAGRAPH 4.4.1.

iii) REVISE REFERENCE FROM "STRUCTURAL DESIGN DRAWINGS" TO "CONTRACT DOCUMENTS" IN PARAGRAPH 3.1.

2. SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS. STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING REQUIREMENTS:

TYPE OF MEMBER

ASTM SPECIFICATION

Fy

WIDE FLANGE SHAPES

A992

50 KSI

OTHER SHAPES, PLATES, AND RODS

A36

36 KSI

OTHER SHAPES AND PLATES

A572 (GRADE 50)

50 KSI

(NOTED GRADE 50 ON PLANS)

A572 (E OR S, GR.B)

35 KSI

PIPE COLUMNS

A500 (GRADE B)

46 KSI

STRUCTURAL TUBING

SQUARE OR RECTANGULAR

46 KSI

ROUND

42 KSI

CONNECTION BOLTS

A325-N

3. ARCHITECTURALLY EXPOSED STRUCTURAL STEEL SHALL CONFORM TO SECTION 10 OF THE AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES.

4. ALL A-325N CONNECTION BOLTS NEED ONLY BE TIGHTENED TO A SNUG TIGHT CONDITION, DEFINED AS THE TIGHTNESS THAT EXISTS WHEN ALL PLIES IN A JOINT ARE IN FIRM CONTACT. THIS MAY BE ATTAINED BY A FEW IMPACTS OF AN IMPACT WRENCH OR THE FULL EFFORT OF AN IRONWORKER USING AN ORDINARY SPUD WRENCH.

5. ALL ANCHORS EMBEDDED IN CONCRETE SHALL BE A307 HEADED BOLTS OR A36 THREADED ROD WITH AN ASTM 563 HEAVY HEX NUT TACK WELDED ON THE EMBEDDED END.

6. ALL WELDING SHALL BE IN CONFORMANCE WITH A.I.C.C. AND A.W.S. STANDARDS AND SHALL BE PERFORMED BY W.A.B.O. CERTIFIED WELDERS USING E70 XX ELECTRODES. ONLY PREQUALIFIED WELDS (AS DEFINED BY A.W.S.) SHALL BE USED. ALL COMPLETE JOINT PENETRATION GROOVE WELDS SHALL BE MADE WITH A FILLER MATERIAL THAT HAS A MINIMUM CVN TOUGHNESS OF 20 FT-LBS AT -20 DEGREES F AND 40 FT-LBS AT 70 DEGREES F, AS DETERMINED BY AWS CLASSIFICATION OR MANUFACTURER CERTIFICATION.

Wood

1. FRAMING LUMBER SHALL BE KILN DRIED OR MC-19, AND GRADED AND MARKED IN CONFORMANCE WITH W.C.L.B. STANDARD GRADING RULES FOR WEST COAST LUMBER NO.17. FURNISH TO THE FOLLOWING MINIMUM STANDARDS:

JOISTS

(2X & 3X MEMBERS)

HEM-FIR NO. 2

AND BEAMS:

(4X MEMBERS)

DOUGLAS FIR-LARCH NO. 2

MINIMUM BASE VALUE, Fb=850 PSI

BEAMS:

(INCL. 6X AND LARGER)

DOUGLAS FIR-LARCH NO. 1

MINIMUM BASE VALUE, Fb=1350 PSI

POSTS:

(4X MEMBERS)

DOUGLAS FIR-LARCH NO. 2

MINIMUM BASE VALUE, Fc=1350 PSI

(6X AND LARGER)

DOUGLAS FIR-LARCH NO. 1

MINIMUM BASE VALUE, Fc=1000 PSI

STUDS, PLATES & MISC. FRAMING:

DOUGLAS-FIR-LARCH OR HEM-FIR NO. 2

2. GLUED LAMINATED MEMBERS SHALL BE FABRICATED IN CONFORMANCE WITH ASTM AND AITC STANDARDS. EACH MEMBER SHALL BEAR AN A.I.T.C. IDENTIFICATION MARK AND SHALL BE ACCOMPANIED BY AN A.I.T.C. CERTIFICATE OF CONFORMANCE. ALL SIMPLE SPAN BEAMS SHALL BE DOUGLAS FIR COMBINATION 24F-V4, Fb=2,400 PSI, Fv=265 PSI. ALL CANTILEVERED BEAMS SHALL BE DOUGLAS FIR COMBINATION 24F-V8, Fb=2400 PSI, Fv=265 PSI. CAMBER ALL SIMPLE SPAN GLULAM BEAMS TO 3,000" RADIUS, UNLESS SHOWN OTHERWISE ON THE PLANS.

3. MANUFACTURED LUMBER, PSL, LVL AND LSL SHALL BE MANUFACTURED UNDER A PROCESS APPROVED BY THE NATIONAL RESEARCH BOARD. EACH PIECE SHALL BEAR A STAMP OR STAMPS NOTING THE NAME AND PLANT NUMBER OF THE MANUFACTURER, THE GRADE, THE NATIONAL RESEARCH BOARD NUMBER, AND THE QUALITY CONTROL AGENCY. ALL PSL, LVL, AND LSL LUMBER SHALL BE MANUFACTURED IN ACCORDANCE WITH ICC-ES REPORT ESR-1387 USING DOUGLAS FIR VENEER GLUED WITH A WATERPROOF ADHESIVE MEETING THE REQUIREMENTS OF ASTM D2559 WITH ALL GRAIN PARALLEL WITH THE LENGTH OF THE MEMBER. THE MEMBERS SHALL HAVE THE FOLLOWING MINIMUM PROPERTIES:

PSL (2.0E)

Fb=2900 PSI, E=2000 KSI, Fv=290 PSI

LVL (1.9E)

Fb=2600 PSI, E=1900 KSI, Fv=285 PSI

LSL (1.55E)

Fb=2325 PSI, E=1550 KSI, Fv=310 PSI

DESIGN SHOWN ON PLANS IS BASED ON LUMBER MANUFACTURED BY THE WEYERHAEUSER CORPORATION. ALTERNATE MANUFACTURERS MAY BE USED SUBJECT TO REVIEW AND APPROVAL BY THE ARCHITECT AND STRUCTURAL ENGINEER, ALTERNATE JOIST HANGERS AND OTHER HARDWARE MAY BE SUBSTITUTED FOR ITEMS SHOWN PROVIDED THEY HAVE ICC APPROVAL FOR EQUAL OR GREATER LOAD CAPACITIES. ALL JOIST HANGERS AND OTHER HARDWARE SHALL BE COMPATIBLE IN SIZE WITH MEMBERS PROVIDED. MANUFACTURED LUMBER PRODUCTS SHALL BE INSTALLED WITH A MOISTURE CONTENT OF 12% OR LESS. THE CONTRACTOR SHALL MAKE PROVISIONS DURING CONSTRUCTION TO PREVENT THE MOISTURE CONTENT OF INSTALLED BEAMS FROM EXCEEDING 12%. EXCESSIVE DEFLECTIONS MAY OCCUR IF MOISTURE CONTENT EXCEEDS THIS VALUE.

4. PREFABRICATED PLYWOOD WEB JOIST DESIGN SHOWN ON PLANS IS BASED ON JOISTS MANUFACTURED BY THE TRUS-JOIST CORPORATION, ALTERNATE PLYWOOD WEB JOIST MANUFACTURERS MAY BE USED SUBJECT TO REVIEW AND APPROVAL BY THE ARCHITECT AND STRUCTURAL ENGINEER. ALTERNATE JOIST HANGERS AND OTHER HARDWARE MAY BE SUBSTITUTED FOR ITEMS SHOWN PROVIDED THEY HAVE I.C.B.O. APPROVAL FOR EQUAL OR GREATER LOAD CAPACITIES. ALL JOIST HANGERS AND OTHER HARDWARE SHALL BE COMPATIBLE IN SIZE WITH PLYWOOD WEB JOIST PROVIDED.

Wood (Continued)

5. PREFABRICATED CONNECTOR PLATE WOOD ROOF TRUSSES SHALL BE DESIGNED BY THE MANUFACTURER IN ACCORDANCE WITH THE "NATIONAL DESIGN STANDARD FOR METAL PLATE-CONNECTED WOOD TRUSS CONSTRUCTION, ANSI/TPI 1" BY THE TRUSS PLATE INSTITUTE FOR THE SPANS AND CONDITIONS SHOWN ON THE PLANS. LOADING SHALL BE AS FOLLOWS:

TOP CHORD LIVE LOAD

25 PSF

TOP CHORD DEAD LOAD

10 PSF

BOTTOM CHORD DEAD LOAD

5 PSF

TOTAL LOAD

40 PSF

WIND UPLIFT (TOP CHORD)

5 PSF

BOTTOM CHORD LIVE LOAD

10 PSF

BOTTOM CHORD LIVE LOAD DOES NOT ACT CONCURRENTLY WITH THE ROOF LIVE LOAD)

WOOD TRUSSES SHALL UTILIZE APPROVED CONNECTOR PLATES (GANGNAIL OR EQUAL). SUBMIT SHOP DRAWINGS AND DESIGN CALCULATIONS TO THE ARCHITECT AND STRUCTURAL ENGINEER FOR REVIEW PRIOR TO FABRICATION. SUBMITTED DOCUMENTS SHALL BE SIGNED AND STAMPED BY A STRUCTURAL ENGINEER REGISTERED IN THE STATE OF WASHINGTON. PROVIDE FOR SHAPES, BEARING POINTS, INTERSECTIONS, HIPS, VALLEYS, ETC., SHOWN ON THE DRAWINGS. THE EXACT COMPOSITION OF SPECIAL HIP, VALLEY, AND INTERSECTION AREAS (USE OF GIRDER TRUSSES, JACK TRUSSES, STEP-DOWN TRUSSES, ETC.) SHALL BE DETERMINED BY THE MANUFACTURER UNLESS SPECIFICALLY INDICATED ON THE PLANS. PROVIDE ALL TRUSS TO TRUSS AND TRUSS TO GIRDER TRUSS CONNECTION DETAILS AND REQUIRED CONNECTION MATERIALS. PROVIDE FOR ALL TEMPORARY AND PERMANENT TRUSS BRACING AND BRIDGING.

6. PLYWOOD SHEATHING SHALL BE GRADE C-D, EXTERIOR GLUE OR STRUCTURAL II, EXTERIOR GLUE IN CONFORMANCE WITH DOC PS 1. ORIENTED STRAND BOARD OF EQUIVALENT THICKNESS, EXPOSURE RATING AND PANEL INDEX MAY BE USED IN LIEU OF PLYWOOD.

A. ROOF SHEATHING SHALL BE 5/8" (NOMINAL) WITH SPAN RATING 40/20.

B. FLOOR SHEATHING SHALL BE 3/4" (NOMINAL) WITH SPAN RATING 48/24.

C. WALL SHEATHING SHALL BE 1/2" (NOMINAL) WITH SPAN RATING 24/0.

D. REFER TO WOOD FRAMING NOTES BELOW FOR TYPICAL NAILING REQUIREMENTS.

7. ALL WOOD IN DIRECT CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESSURE-TREATED WITH AN APPROVED PRESERVATIVE OR (2) LAYERS OF ASPHALT IMPREGNATED BUILDING PAPER SHALL BE PROVIDED BETWEEN UNTREATED WOOD AND CONCRETE OR MASONRY. PRESSURE TREATED WOOD SHALL BE TREATED PER AWPFA STANDARD. PRESSURE TREATED WOOD FOR ABOVE GROUND USE SHALL BE TREATED TO A RETENTION OF 0.25 PCF. WOOD IN CONTINUOUS CONTACT WITH FRESH WATER OR SOIL SHALL BE TREATED TO A RETENTION OF 0.40 PCF. WOOD FOR USE IN PERMANENT FOUNDATIONS SHALL BE TREATED TO A RETENTION OF 0.60 PCF. SODIUM BORATE (SBX) TREATED WOOD SHALL NOT BE USED WHERE EXPOSED TO WEATHER. FASTENERS AND TIMBER CONNECTORS IN DIRECT CONTACT WITH ACQ-A, CBA-A, CA-B, OR SBX TREATED WOOD SHALL BE G185 OR A185 HOT DIPPED OR CONTINUOUS HOT-GALVANIZED PER ASTM A653. FASTENERS AND TIMBER CONNECTORS IN DIRECT CONTACT WITH ACZA TREATED WOOD SHALL BE TYPE 304 OR 316 STAINLESS STEEL.

9. TIMBER CONNECTORS CALLED OUT BY LETTERS AND NUMBERS SHALL BE "STRONG-TIE" BY SIMPSON COMPANY, AS SPECIFIED IN THEIR CATALOG NUMBER C-C-2024. EQUIVALENT DEVICES BY OTHER MANUFACTURERS MAY BE SUBSTITUTED, PROVIDED THEY HAVE ICC-ES APPROVAL FOR EQUAL OR GREATER LOAD CAPACITIES. PROVIDE NUMBER AND SIZE OF FASTENERS AS SPECIFIED BY MANUFACTURER. CONNECTORS SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. ALL 2X JOISTS SHALL BE CONNECTED TO FLUSH BEAMS WITH "LUS" SERIES JOIST HANGERS. ALL TJ JOISTS SHALL BE CONNECTED TO FLUSH BEAMS WITH "IUS" SERIES JOIST HANGERS. ALL DOUBLE JOIST BEAMS SHALL BE CONNECTED TO FLUSH BEAMS WITH "MIT" SERIES JOIST HANGERS. WHERE CONNECTOR STRAPS CONNECT TWO MEMBERS, PLACE ONE-HALF OF THE NAILS OR BOLTS IN EACH MEMBER. ALL SHIMS SHALL BE SEASONED AND DRIED AND THE SAME GRADE (MINIMUM) AS MEMBERS CONNECTED.

10. WOOD FASTENERS

A. NAIL SIZES SPECIFIED ON DRAWINGS ARE BASED ON THE FOLLOWING SPECIFICATIONS:

SIZE	LENGTH	DIAMETER
6d	2"	0.113"
8d	2-1/2"	0.131"
10d	3"	0.148"
12d	3-1/4"	0.148"
16d BOX	3-1/2"	0.135"

IF CONTRACTOR PROPOSES THE USE OF ALTERNATE NAILS, THEY SHALL SUBMIT NAIL SPECIFICATIONS TO THE STRUCTURAL ENGINEER (PRIOR TO CONSTRUCTION) FOR REVIEW AND APPROVAL.

NAILS - PLYWOOD (APA RATED SHEATHING) FASTENERS TO FRAMING SHALL BE DRIVEN FLUSH TO FACE OF SHEATHING WITH NO COUNTERSINKING PERMITTED.

B. ALL BOLTS IN WOOD MEMBERS SHALL CONFORM TO ASTM A307. PROVIDE WASHERS UNDER THE HEADS AND NUTS OF ALL BOLTS AND LAG BOLTS BEARING ON WOOD. INSTALLATION OF LAG BOLTS SHALL CONFORM TO THE NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION (2021 EDITION) WITH A LEAD BORE HOLE OF 60 TO 70 PERCENT OF THE SHANK DIAMETER. LEAD HOLES ARE NOT REQUIRED FOR 3/8" AND SMALLER LAG SCREWS.

11. WOOD FRAMING NOTES--THE FOLLOWING APPLY AT AREAS OF NEW CONSTRUCTION UNLESS OTHERWISE SHOWN ON THE PLANS:

A. ALL WOOD FRAMING DETAILS NOT SHOWN OTHERWISE SHALL BE CONSTRUCTED TO THE MINIMUM STANDARDS OF THE INTERNATIONAL BUILDING CODE. MINIMUM NAILING UNLESS OTHERWISE NOTED, SHALL CONFORM TO TABLE 2304.10.2. COORDINATE THE SIZE AND LOCATION OF ALL OPENINGS WITH MECHANICAL AND ARCHITECTURAL DRAWINGS.

B. WALL FRAMING: REFER ARCHITECTURAL DRAWINGS FOR THE SIZE OF ALL WALLS. ALL STUDS SHALL BE SPACED AT 16" O.C. UNO. TWO STUDS MINIMUM SHALL BE PROVIDED AT THE END OF ALL WALLS AND AT EACH SIDE OF ALL OPENINGS, AND AT BEAM OR HEADER BEARING LOCATIONS. TWO 2x8 HEADERS SHALL BE PROVIDED OVER ALL OPENINGS NOT OTHERWISE NOTED. SOLID BLOCKING FOR WOOD COLUMNS SHALL BE PROVIDED THROUGH FLOORS TO SUPPORTS BELOW. PROVIDE CONTINUOUS SOLID BLOCKING AT MID-HEIGHT OF ALL STUD WALLS OVER 10'-0" IN HEIGHT. ALL WALLS SHALL HAVE A SINGLE BOTTOM PLATE AND A DOUBLE TOP PLATE. END NAIL TOP PLATE TO EACH STUD WITH TWO 16d NAILS, AND TOENAIL OR END NAIL EACH STUD TO BOTTOM PLATE WITH TWO 16d NAILS. FACE NAIL DOUBLE TOP PLATE WITH 16d @ 12" O.C. AND LAP MINIMUM 4'-0" AT JOINTS AND PROVIDE EIGHT 16d NAILS @ 4" O.C. EACH SIDE JOINT. ALL STUD WALLS SHALL HAVE THEIR LOWER WOOD PLATES ATTACHED TO WOOD FRAMING BELOW WITH TWO ROWS OF 16d nails @ 12" ON-CENTER, OR ATTACHED TO CONCRETE BELOW WITH 5/8" DIAMETER ANCHOR BOLTS @ 4'-0" ON-CENTER EMBEDDED 7" MINIMUM, UNLESS INDICATED OTHERWISE. INDIVIDUAL MEMBERS OF BUILT-UP POSTS SHALL BE NAILED TO EACH OTHER WITH TWO ROWS OF 16d @12" ON-CENTER, UNLESS OTHERWISE NOTED. GYPSUM WALLBOARD SHALL BE FASTENED TO THE INTERIOR SURFACE OF ALL STUDS AND PLATES WITH NO. 6 X 1-1/4" TYPE S OR W SCREWS @ 8" ON-CENTER, UNLESS INDICATED OTHERWISE. 1/2" (NOMINAL) APA RATED SHEATHING (SPAN RATING 24/0) SHALL BE NAILED TO ALL EXTERIOR SURFACES WITH 8d NAILS @ 6" ON-CENTER AT PANEL EDGES AND TOP AND BOTTOM PLATES (BLOCK UN-SUPPORTED EDGES) AND TO ALL INTERMEDIATE STUDS AND BLOCKING WITH 8d NAILS @ 12" ON-CENTER ALLOW 1/8" SPACING AT ALL PANEL EDGES AND PANEL ENDS.

C. FLOOR AND ROOF FRAMING: PROVIDE DOUBLE JOISTS UNDER ALL PARALLEL PARTITIONS THAT EXTEND OVER MORE THAN HALF THE JOIST LENGTH AND AROUND ALL OPENINGS IN FLOORS OR ROOFS UNLESS OTHERWISE NOTED. PROVIDE SOLID BLOCKING AT ALL BEARING POINTS. TOENAIL JOISTS TO SUPPORTS WITH TWO 16d NAILS. ATTACH TIMBER JOISTS TO FLUSH HEADERS OR BEAMS WITH SIMPSON METAL JOIST HANGERS IN ACCORDANCE WITH NOTES ABOVE. NAIL ALL MULTI-JOIST BEAMS TOGETHER WITH TWO ROWS 16d @ 12" ON-CENTER.

UNLESS OTHERWISE NOTED ON THE PLANS, PLYWOOD ROOF AND FLOOR SHEATHING SHALL BE LAID UP WITH GRAIN PERPENDICULAR TO SUPPORTS AND NAILED AT 6" ON-CENTER WITH 8d NAILS TO FRAMED PANEL EDGES, STRUTS AND OVER STUD WALLS AS SHOWN ON PLANS AND @ 12" ON-CENTER TO INTERMEDIATE SUPPORTS. PROVIDE APPROVED PLYWOOD EDGE CLIPS CENTERED BETWEEN JOISTS/TRUSSES AT UNBLOCKED ROOF SHEATHING EDGES. ALL FLOOR SHEATHING EDGES SHALL HAVE APPROVED T&G JOINTS OR SHALL BE SUPPORTED WITH SOLID BLOCKING. ALLOW 1/8" SPACING AT ALL PANEL EDGES AND ENDS OF FLOOR AND ROOF SHEATHING. TOENAIL BLOCKING TO SUPPORTS WITH 16d @ 12" ON-CENTER UNLESS OTHERWISE NOTED.

4303 Stone Way N
Seattle, WA 98103
206.258.6333

Alpay Residence

5419 96th Ave SE
Mercer Island, WA 98040

No. Date Issue

11/10/25 Permit

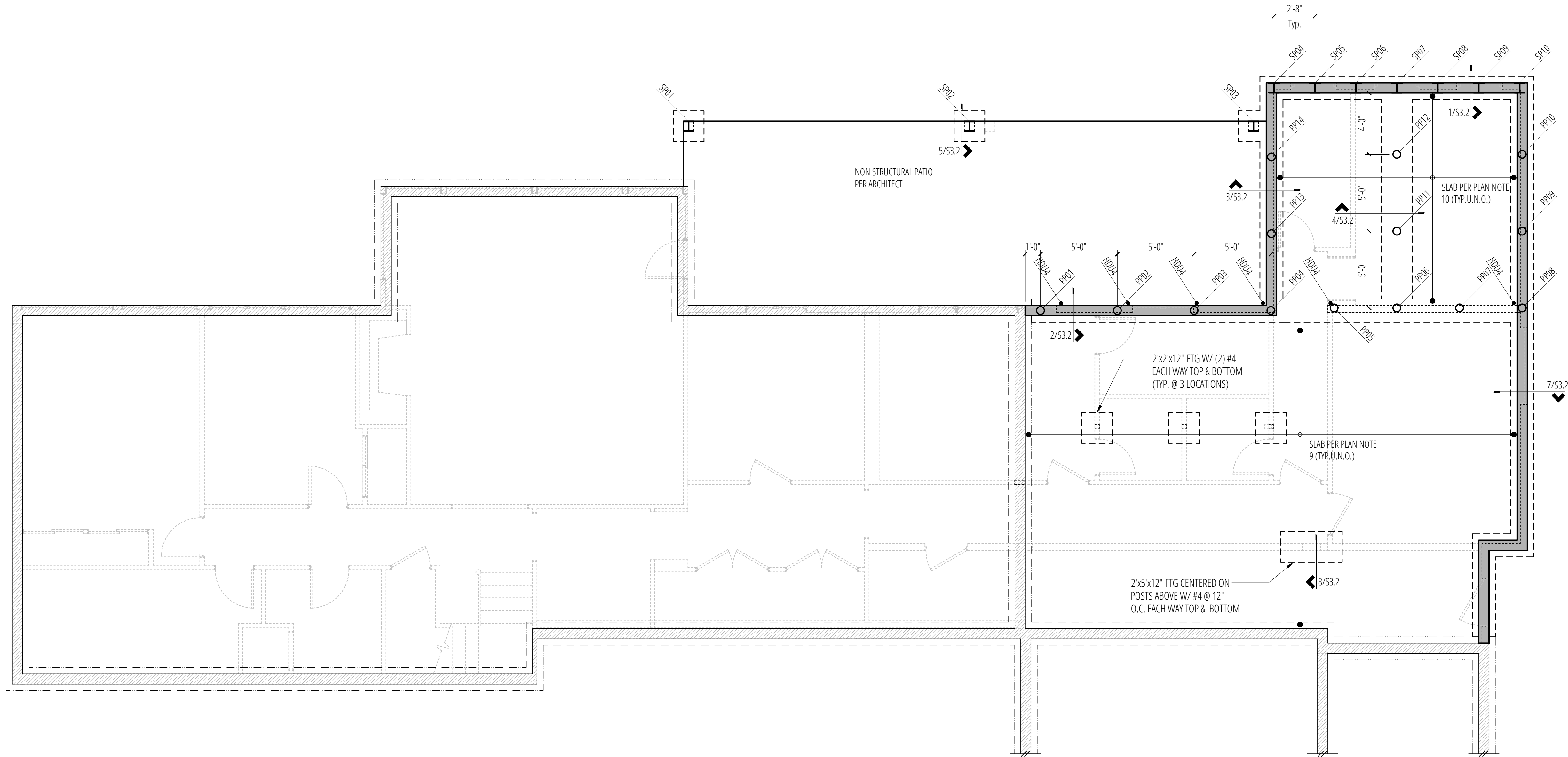
2/26/26 Corrections

Sheet Contents

General Structural Notes

Sheet No.

S1.0



Foundation Plan

SCALE: 1/4"=1'-0"

PLAN NOTES

- REFER TO GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS (S1.1).
- REFER TO ARCHITECTURAL DRAWINGS FOR ALL DIMENSIONS UNLESS SPECIFICALLY NOTED ON STRUCTURAL PLANS.
- ALL NEW FOOTINGS NOT SUPPORTED BY PILES SHALL BEAR ON FIRM, NATIVE SOIL.
- PROVIDE DRAINAGE BEHIND ALL FOUNDATION WALLS.
- REINFORCE FOOTING AND WALL CORNERS AND INTERSECTIONS PER 11/S3.1.
- "HDUX" REFERS TO HOLDDOWNS PER 9/S3.1.
- REFER 4/S3.1 WHERE PIPES PENETRATE FOUNDATION.
- CONTRACTOR TO VERIFY TOP OF FOOTING ELEVATION w/ ARCHITECTURAL PLANS.
- 4" CONCRETE SLAB ON GRADE REINFORCED WITH #3 @ 12" OC EACH WAY, CENTERED IN SLAB. PROVIDE A BASE OF 4" COMPACTED, CLEAN 3/4" MINUS GRAVEL COVERED WITH 6 MIL VAPOR BARRIER. PROVIDE JOINTS PER 7/S3.1.
- 8" CONCRETE SLAB ON GRADE REINFORCED WITH #4 @ 6" OC EACH WAY, TOP & BOTTOM. FILL ANY VOID BETWEEN EXISTING GRADE AND BOTTOM OF SLAB WITH GEOPHAM. PROVIDE A BASE OF 4" COMPACTED, CLEAN 3/4" MINUS GRAVEL COVERED WITH 6 MIL VAPOR BARRIER. FLOOR SHEATHING SHALL BE 3/4" T&G PLYWOOD SHEATHING WITH 48/24 SPAN.

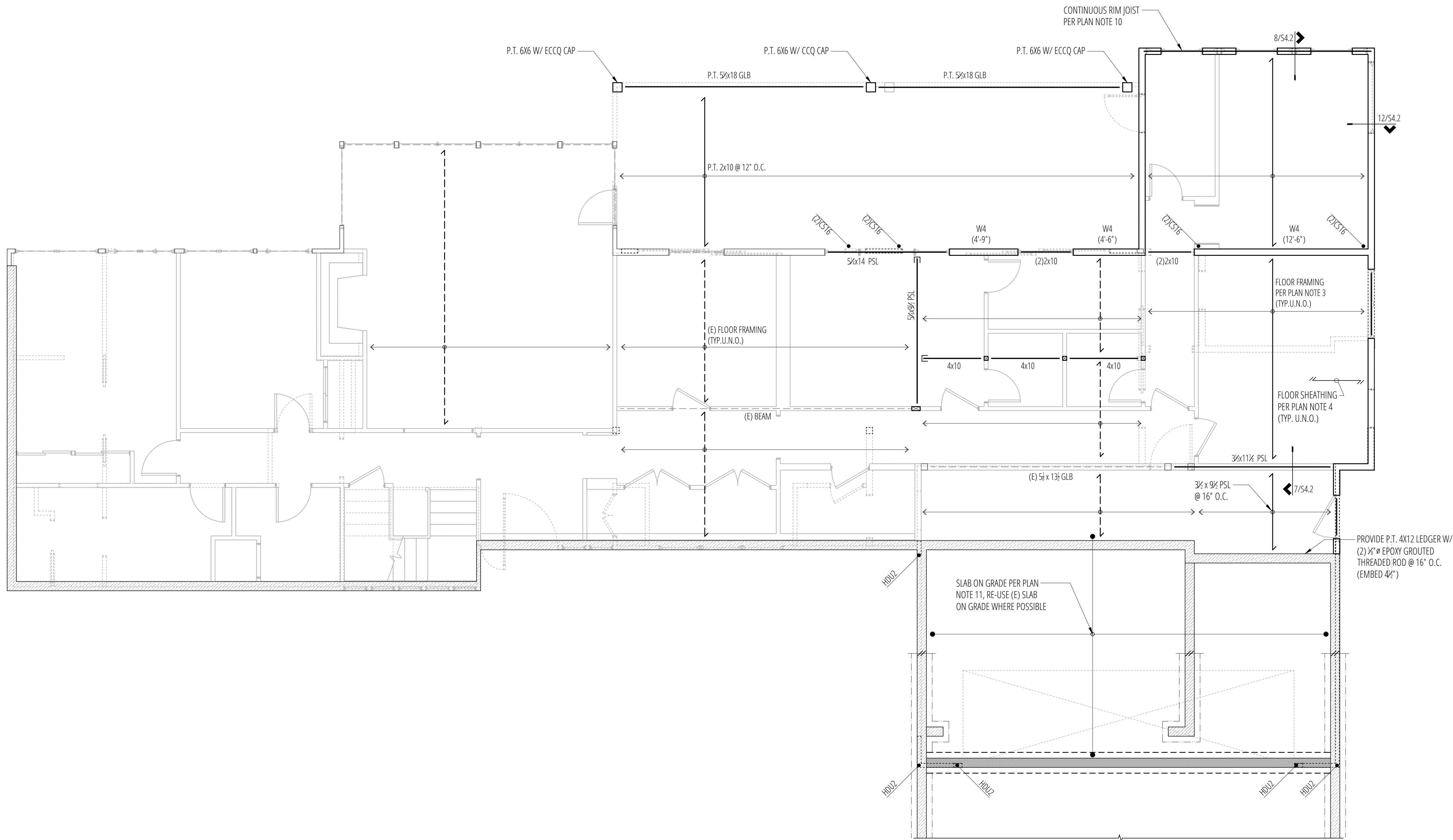
PILE SCHEDULE		
PLAN ID	SHAPE	MINIMUM EMBEDMENT
PP1 - PP14	3" ø STD PIPE	SEE NOTE 1
SP1 - PP10	W8x31	17'-0"

- PIPE PILES SHALL BE DRIVEN TO REFUSAL & INSPECTED PER GENERAL NOTES
- PILES SHALL BE CAPPED w/ MIN. X STL. PL & EMBED 3" MIN - 6" MAX IN CONCRETE FOUNDATIONS
- EXTENT OF PIPE PILE SUPPORT IS APPROXIMATE & SHALL BE VERIFIED BY GEOTECHNICAL ENGINEER IN FIELD. IF ADDITIONAL/LESS PILES ARE REQ'D, COORDINATE REVISIONS w/ ENGINEER OF RECORD.

LEGEND

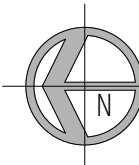
- | | |
|---|--------------------|
| (N) CONCRETE WALL ABOVE THIS LEVEL | (E) SPAN DIRECTION |
| (E) CONCRETE WALL ABOVE THIS LEVEL | SPAN DIRECTION |
| (N) CONCRETE FOOTING | EXTENT OF SPAN |
| (E) CONCRETE FOOTING | JOIST or BEAM |
| STRUCTURAL WOOD WALL or POST BELOW THIS LEVEL | (E) JOIST OR BEAM |
| (E) STRUCTURAL WOOD WALL OR POST BELOW THIS LEVEL | HOLDOWN |
| STRUCTURAL WOOD WALL or POST ABOVE THIS LEVEL | |

No.	Date	Issue
	11/10/25	Permit
Sheet Contents		
FOUNDATION PLAN		
Sheet No.		



Main Floor Framing Plan

SCALE: 1/4"=1'-0"



PLAN NOTES

- REFER TO GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS (S1.1)
- REFER TO ARCHITECTURAL DRAWINGS FOR ALL DIMENSIONS UNLESS SPECIFICALLY NOTED ON STRUCTURAL PLANS.
- NEW FLOOR FRAMING WHERE INDICATED TO BE 11 1/2" TJI 210 @ 16" O.C. (U.N.O.).
- NEW FLOOR SHEATHING SHALL BE 3/4" T&G PLYWOOD SHEATHING WITH 48/24 SPAN RATING. NAIL FRAMED PANEL EDGES W/ 8d COMMON (0.131" DIA. x 2 1/2") @ 6" O.C., FIELD @ 12" O.C. (REFER TO 9/S4.1)
- "W#" REFERS TO SHEARWALL TYPE PER 3/S4.1 & 7/S4.1. ALL OTHER NON-DESIGNATED EXTERIOR WALLS SHALL BE SHEARWALL TYPE W6 WHERE INDICATED. "(x-x)" REFERS TO MINIMUM SHEARWALL LENGTH. COORDINATE ACTUAL LENGTH WITH ARCHITECTURAL.
- "MSTC66/CMST" & "CS16" REFER TO HOLDDOWNS PER 3/S4.2 & 4/S4.2 RESPECTIVELY.
- PROVIDE TOP PLATE SPLICES PER 5/S4.1
- REFER TO 11/S4.1 AT SHEARWALL INTERSECTIONS.
- ALL NEW HEADERS NOT NOTED OTHERWISE ON PLAN SHALL BE (2) 2x10. (REFER TO 6/S4.1)
- AT NEW EXTERIOR WALLS, PROVIDE CONTINUOUS 1 1/2" x 11 1/2" LSL STRUCTURAL RIM JOIST, UNLESS NOTED OTHERWISE. RIM JOISTS OVER OPENINGS SHALL BE CONTINUOUS W/ NO SPLICES. REFER TO 8/S4.2 & 12/S4.2.
- 4" CONCRETE SLAB ON GRADE REINFORCED WITH #3 @ 12" OC EACH WAY, CENTERED IN SLAB. PROVIDE A BASE OF 4" COMPACTED, CLEAN 3/4" MINUS GRAVEL COVERED WITH 6 MIL. VAPOR BARRIER. PROVIDE JOINTS PER 7/S3.1.
- CONTRACTOR TO VERIFY THAT ALL POSTS HAVE CONTINUOUS BEARING THROUGH TO THE FOUNDATION.
- REFER TO DETAIL 1/S3.1 FOR ALL GUARDRAIL REQUIREMENTS.

LEGEND

- | | | | |
|--|---|--|---|
| | STRUCTURAL WOOD WALL or POST BELOW THIS LEVEL | | JOIST or BEAM |
| | STRUCTURAL WOOD WALL or POST ABOVE THIS LEVEL | | (E) JOIST OR BEAM |
| | (E) STRUCTURAL WOOD WALL OR POST BELOW THIS LEVEL | | HOLDOWN |
| | (E) SPAN DIRECTION | | STRAP PER PLAN |
| | SPAN DIRECTION | | BLOCK DIAPHR. 2X'S LAID FLAT @ ALL PANEL EDGES. 8D @ 4" OC @ ALL PANEL EDGES & 12" OC IN FIELD. (REFER TO 9/S4.1) |
| | EXTENT OF SPAN | | |
| | (N) CONCRETE WALL ABOVE THIS LEVEL | | |
| | (E) CONCRETE WALL ABOVE THIS LEVEL | | |

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Sheet Contents		
MAIN FLOOR FRAMING PLAN		
Sheet No.		

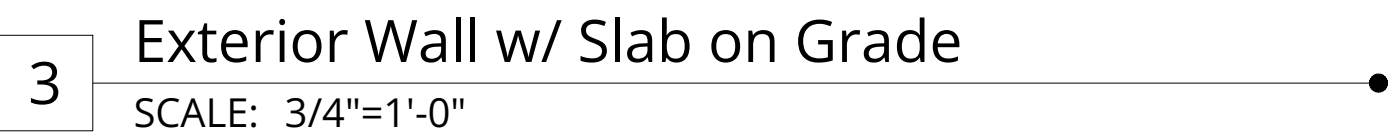
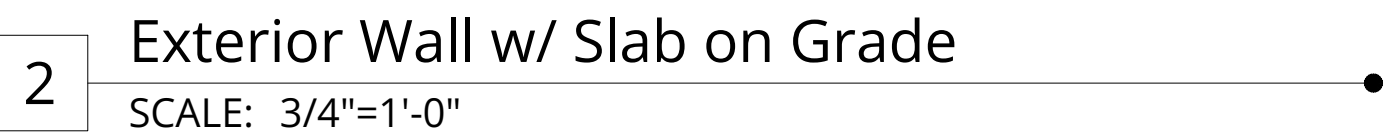
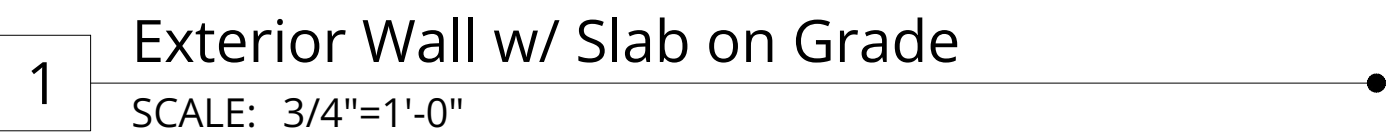
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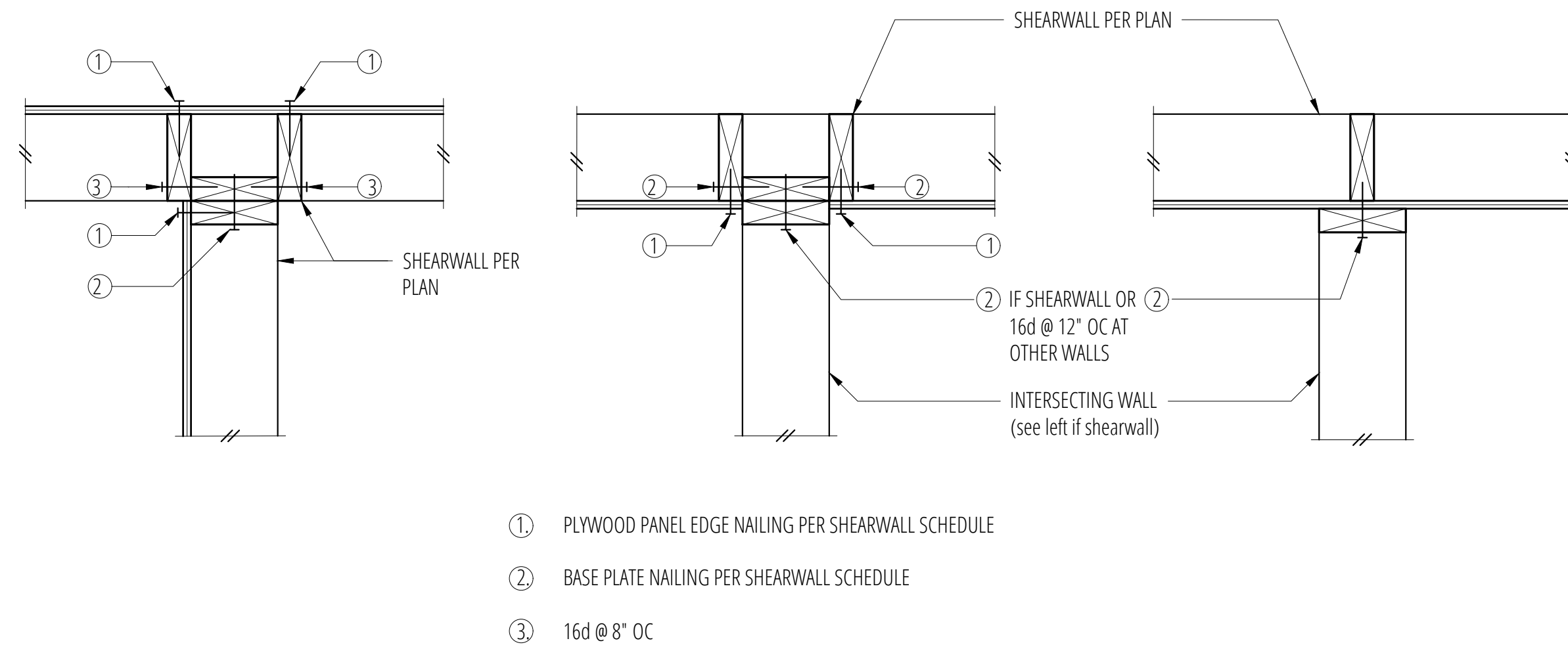
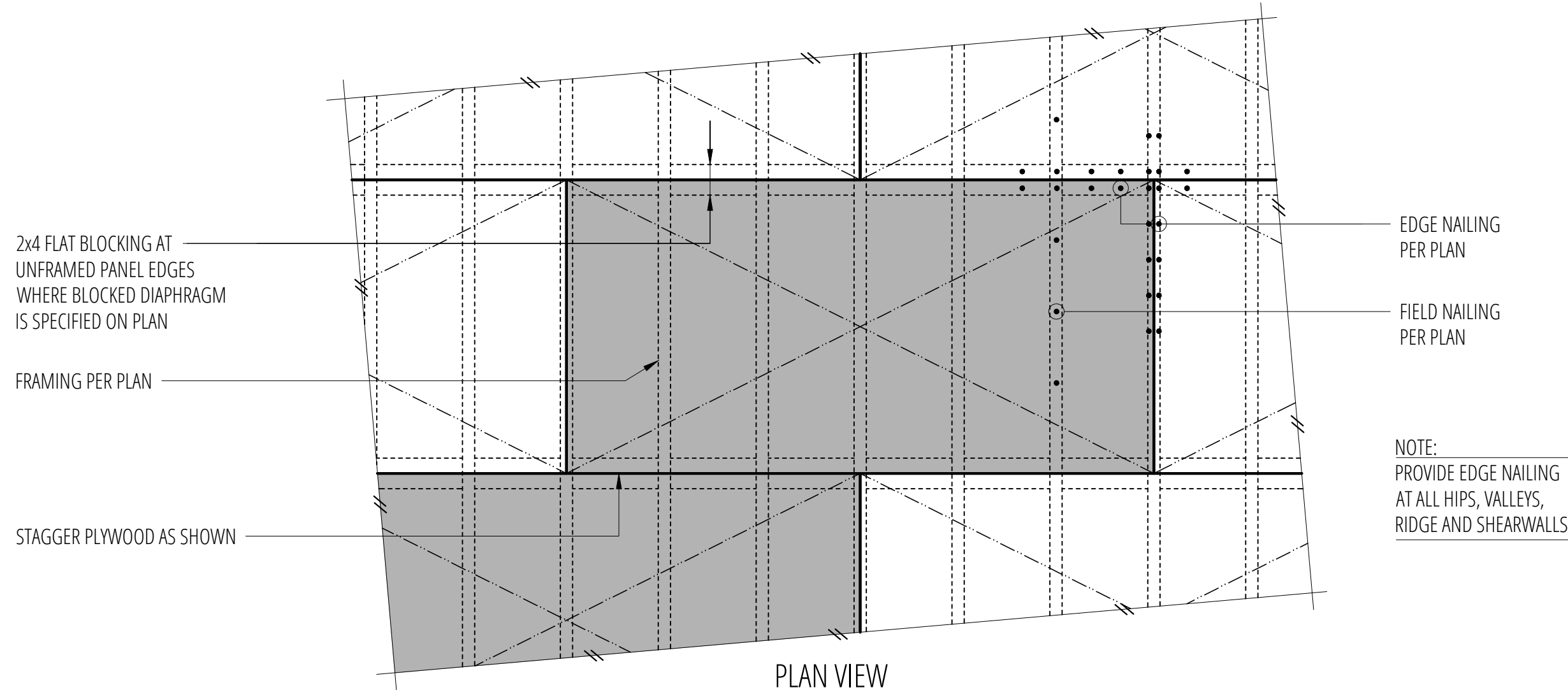
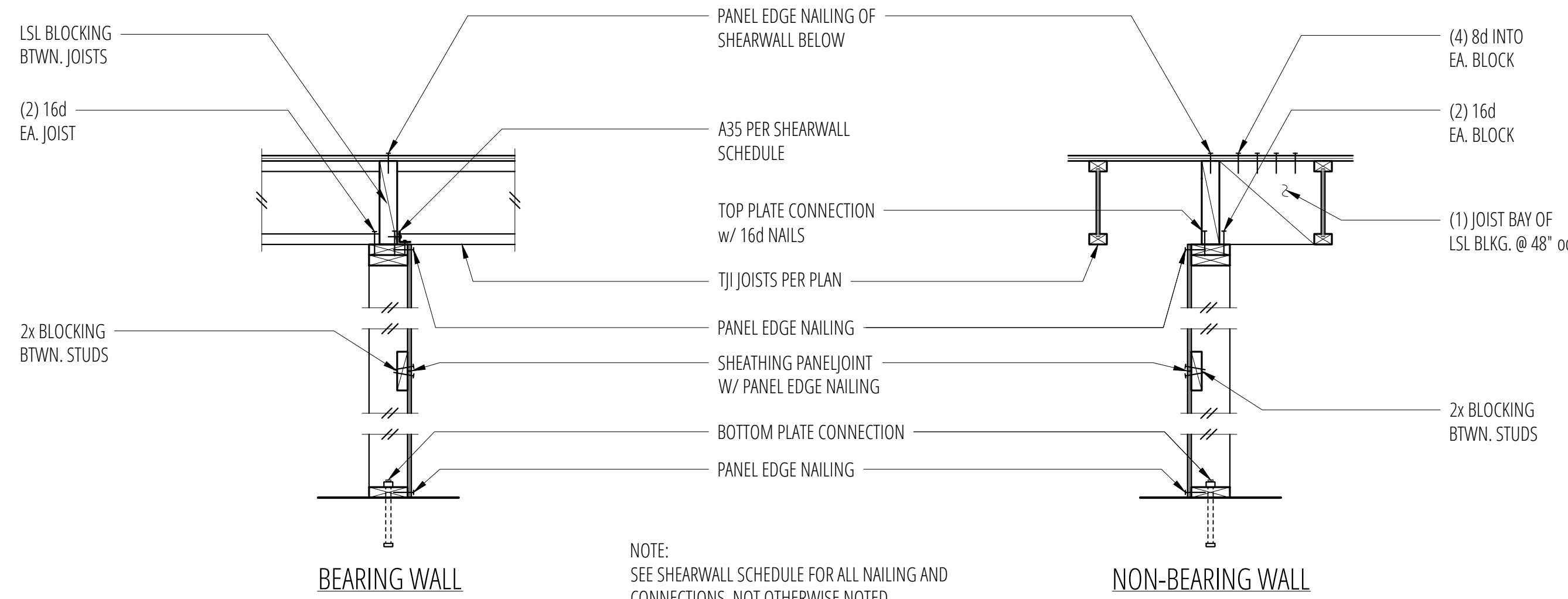
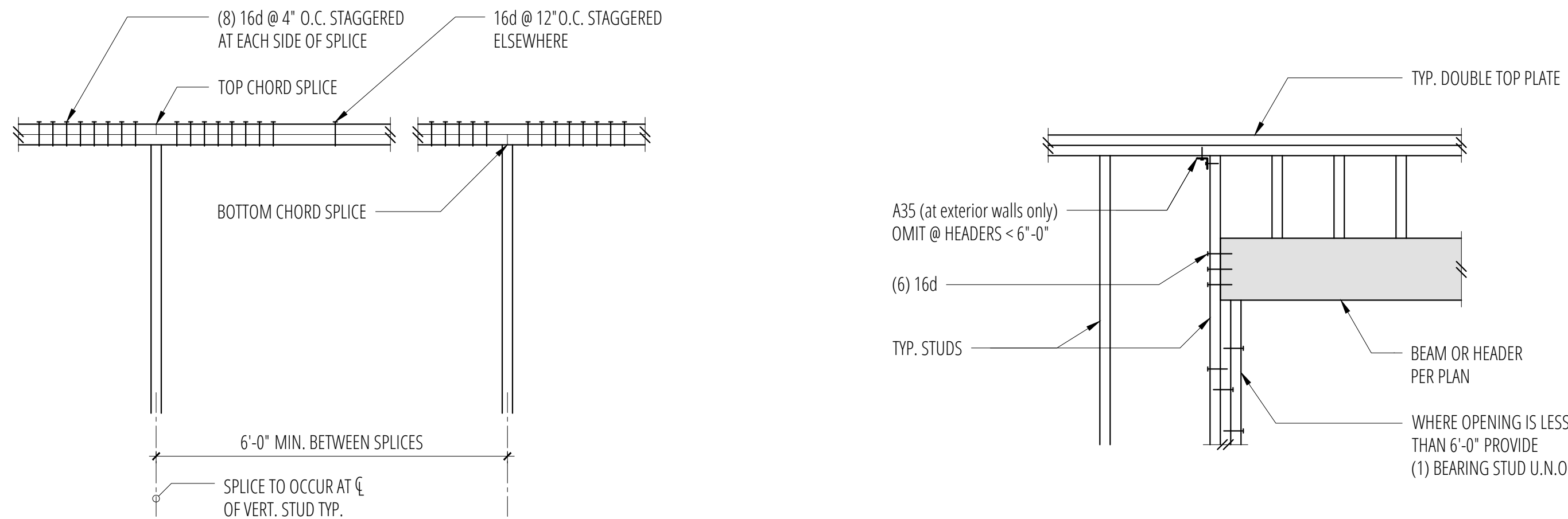
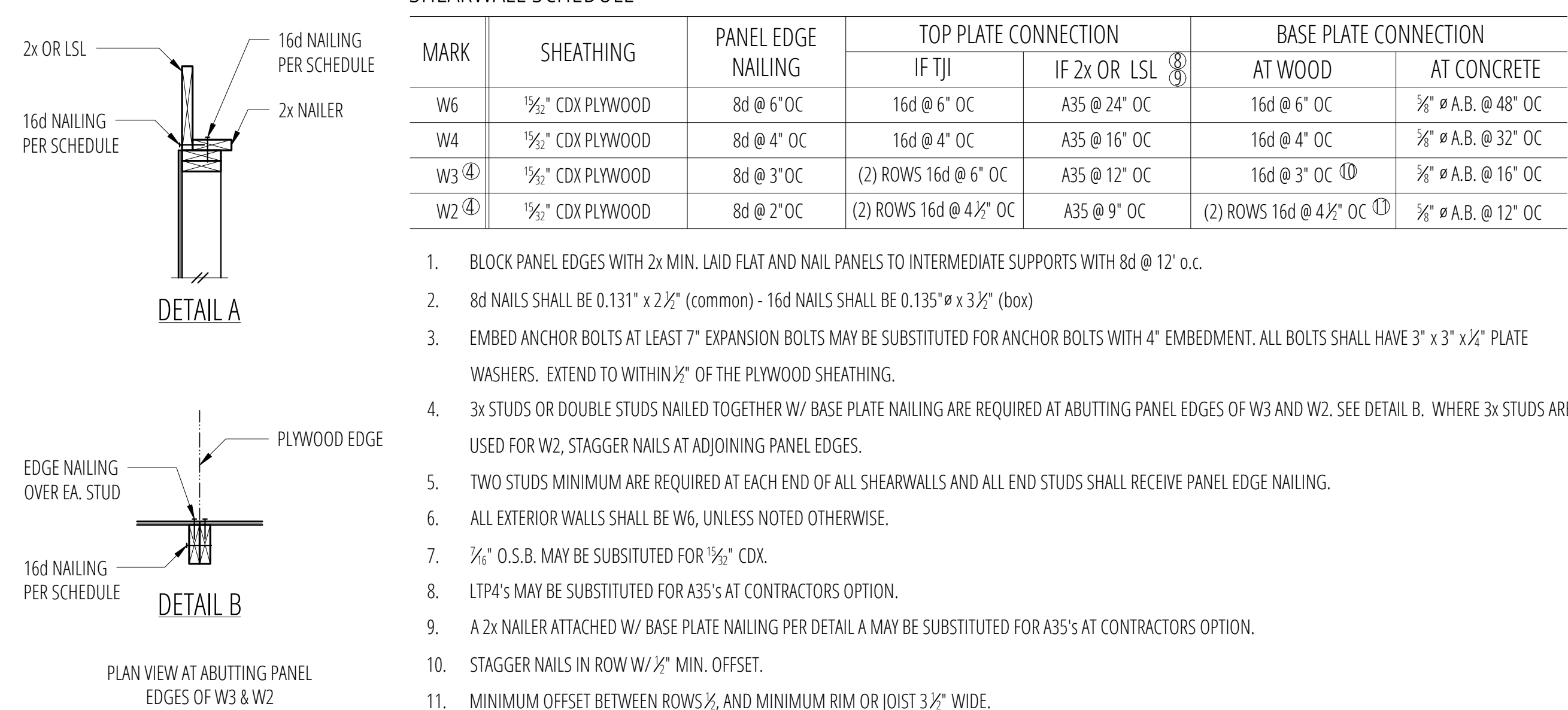
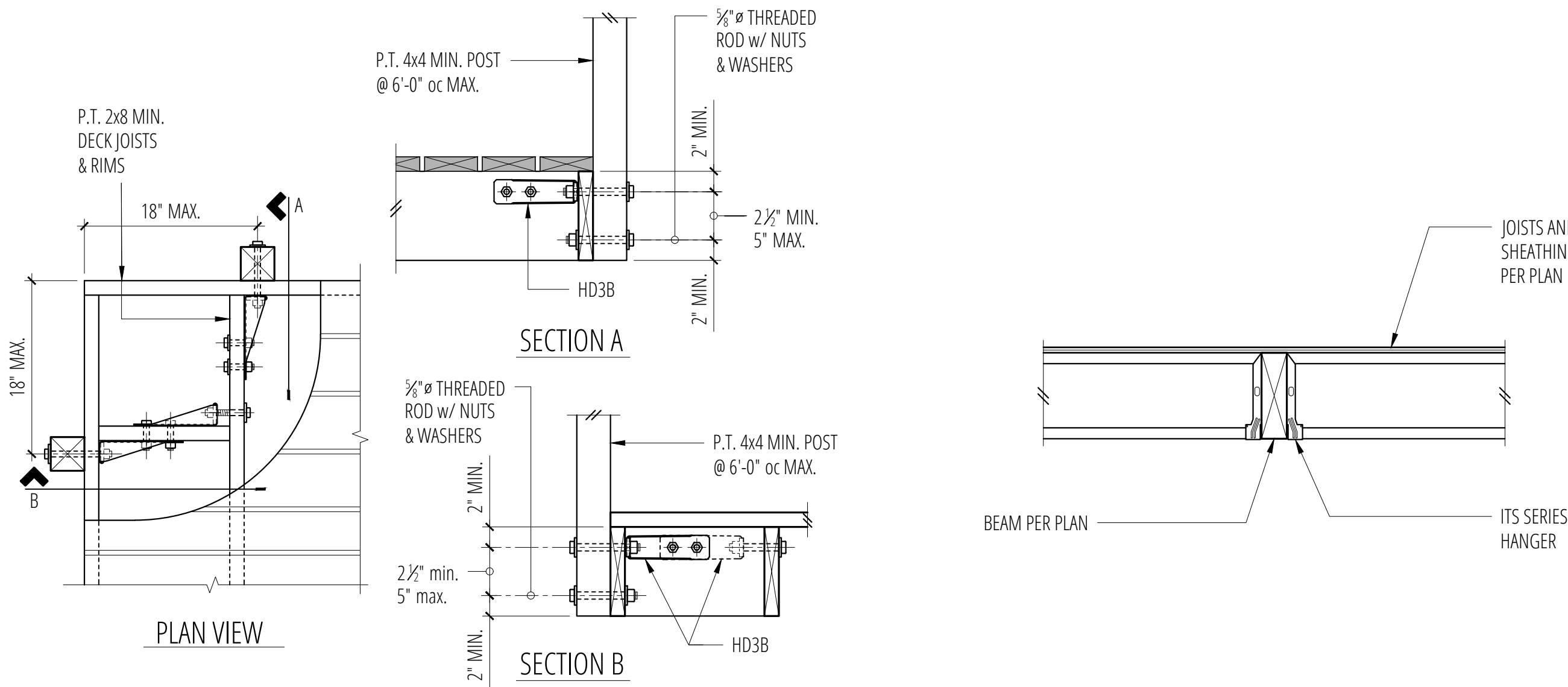
S2.2

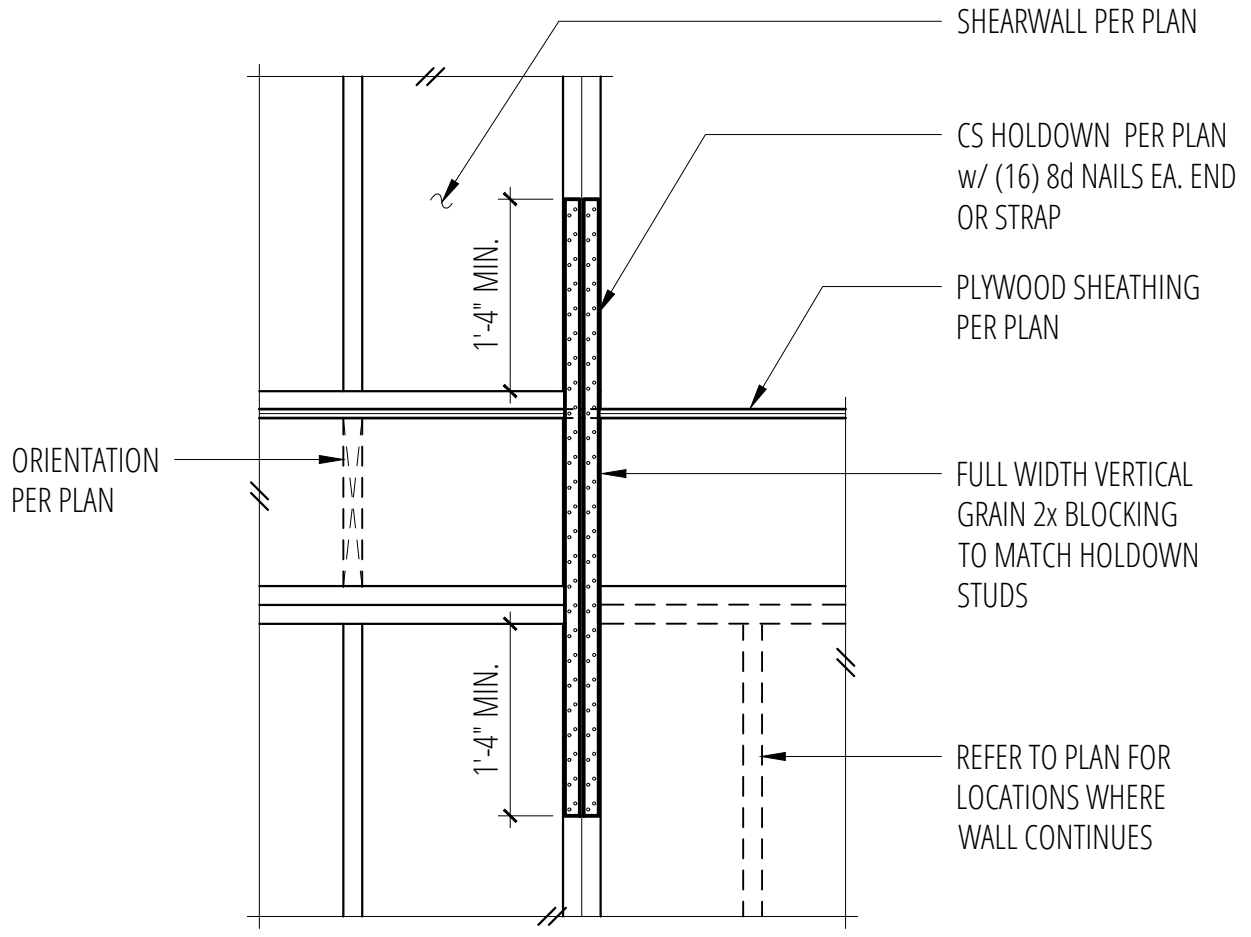
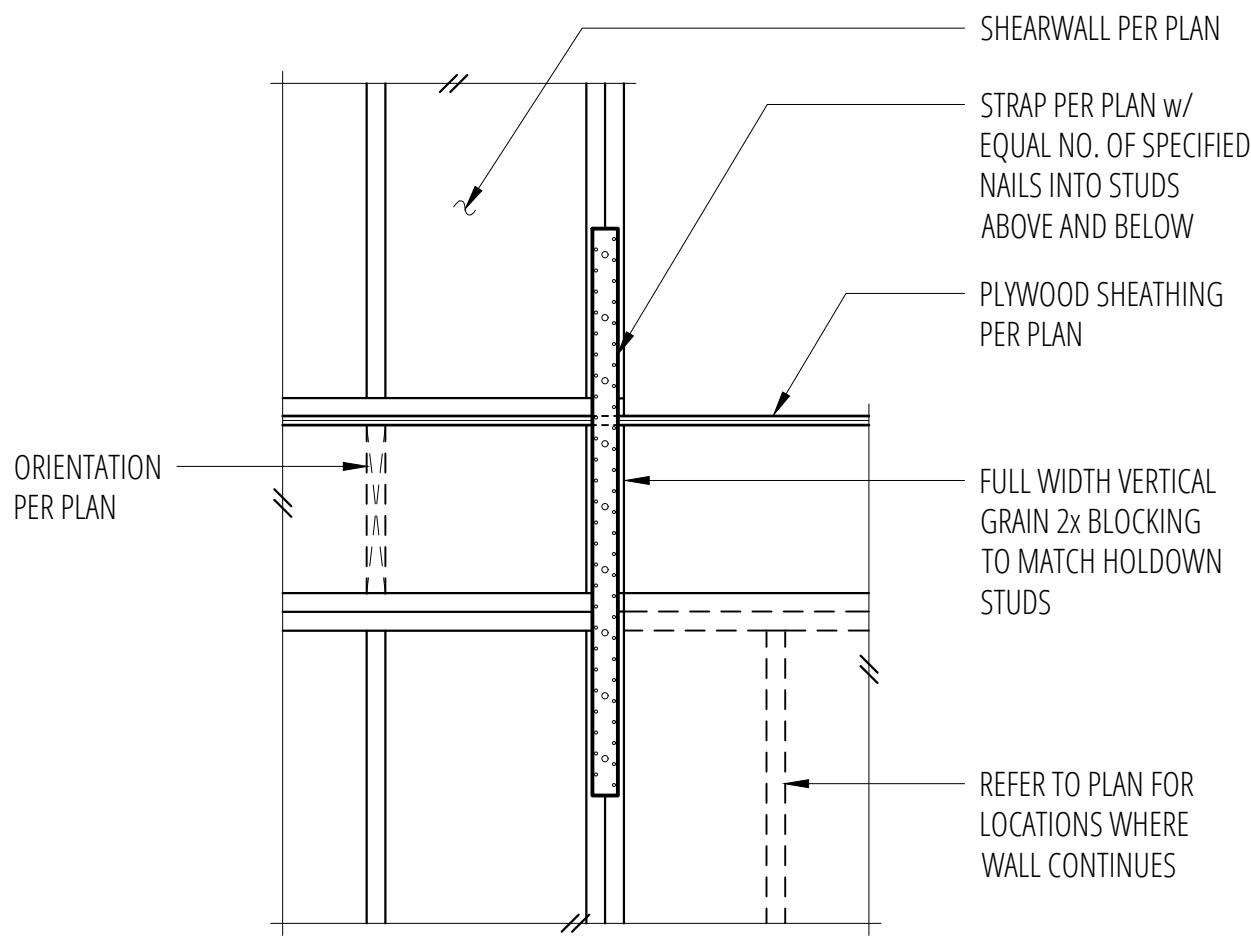


1. REFER TO GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS (S.1.1)
2. REFER TO ARCHITECTURAL DRAWINGS FOR ALL DIMENSIONS UNLESS SPECIFICALLY NOTED ON STRUCTURAL PLANS.
3. NEW ROOF FRAMING SHALL BE PRE-MANUFACTURED ROOF TRUSSES @ 24" O.C. (TRUSS DESIGN BY OTHERS).
4. NEW ROOF SHEATHING SHALL BE 3/4" CDX PLYWOOD SHEATHING WITH 40/20 SPAN RATING. NAIL FRAMED PANEL EDGES W/ 8d COMMON (0.131" DIA. x 2 1/2") @ 6" O.C., FIELD @ 12" O.C. (REFER TO 9/5.4.1)
5. "W" REFERS TO SHEARWALL TYPE PER 3/5.4.1 & 7/5.4.1. ALL OTHER NON-DESIGNATED EXTERIOR WALLS SHALL BE SHEARWALL TYPE W6, WHERE INDICATED. "(x-x)" REFERS TO MINIMUM SHEARWALL LENGTH. COORDINATE ACTUAL LENGTH WITH ARCHITECTURAL.
6. ALL HEADERS AT NEW ROOF NOT NOTED OTHERWISE ON PLAN SHALL BE (2) 2x8. (REFER TO DETAIL 6/5.4.1)
7. PROVIDE TOP PLATE SPLICES PER 5/5.4.1
8. WHERE OVERFRAMING IS INDICATED, OVERFRAME WITH 2x6 @ 24" O.C. w/ 4" MAX SPAN. (REFER TO DETAIL 2/5.1 FOR CONNECTION OF OVERFRAMING TO PRIMARY ROOF).
9. PROVIDE 11/5.4.1 AT SHEARWALL INTERSECTIONS.

	STRUCTURAL WOOD WALL or POST BELOW THIS LEVEL		JOIST or BEAM
	STRUCTURAL WOOD WALL or POST ABOVE THIS LEVEL		(E) JOIST or BEAM
	(E) STRUCTURAL WOOD WALL OR POST BELOW THIS LEVEL		HOLDOWN
	(E) SPAN DIRECTION		STRAP PER PLAN
	SPAN DIRECTION	G.T.	GIRDER TRUSS
	EXTENT OF SPAN	H.T.	HIP TRUSS
	(E) ROOFLINE		OVERFRAMING
	ROOFLINE		BLOCK DIAPH. 2X'S LAID FLAT @ ALL PANE EDGES. 8D @ 4" OC @ ALL PANEL EDGES & 12" OC IN FIELD. (REFER TO 9/54.)





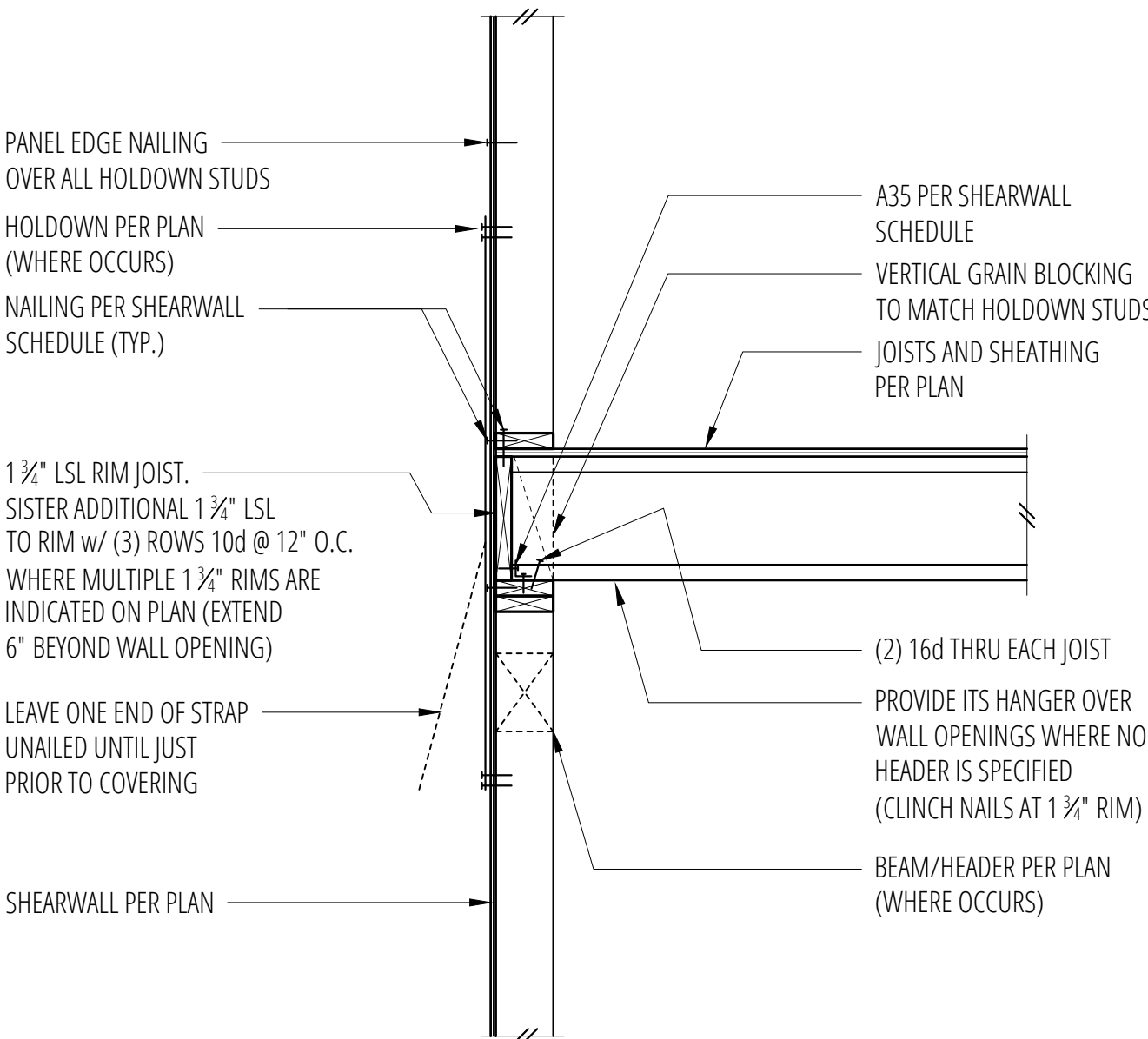
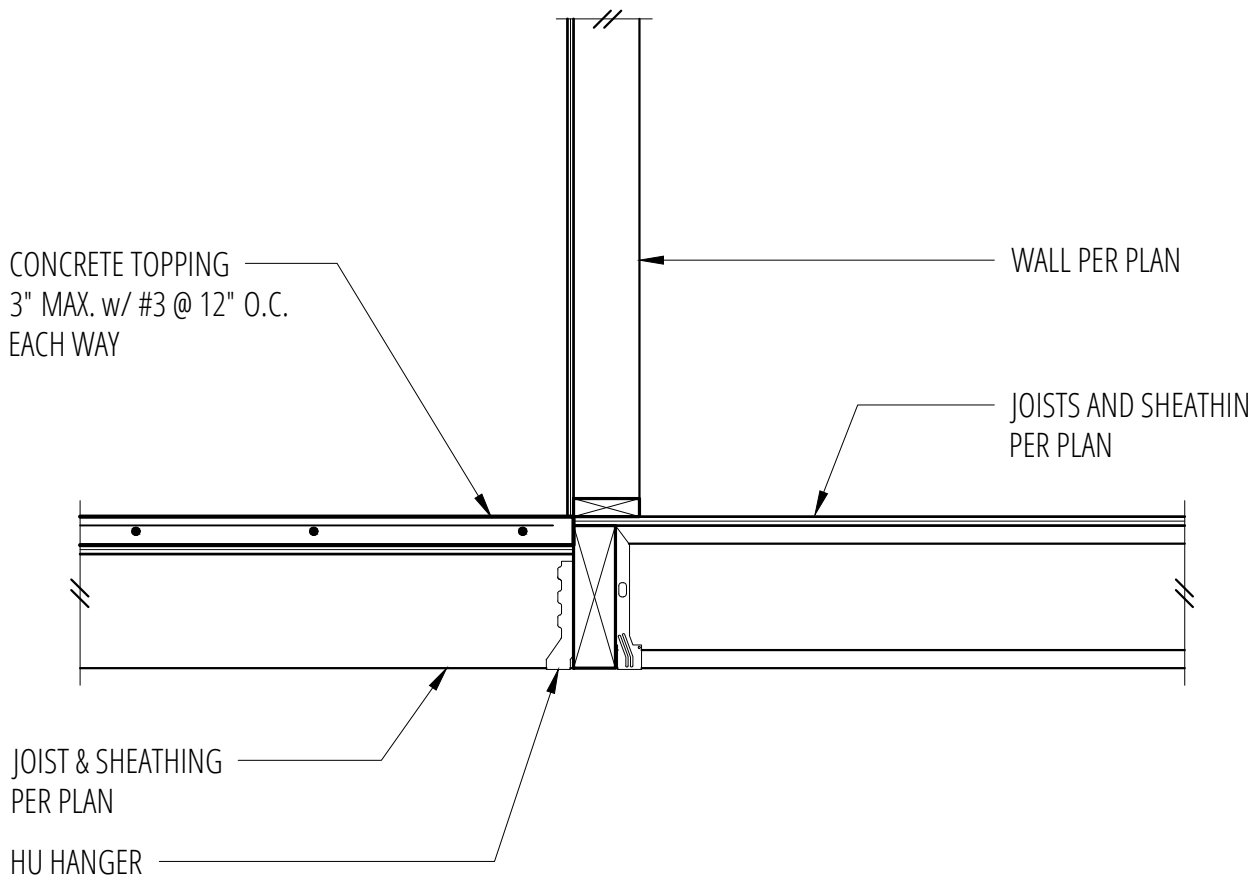


1 SCALE: 3/4"=1'-0"

2 SCALE: 3/4"=1'-0"

3 Typical MST/MSTC Holdown at Floor
SCALE: 3/4"=1'-0"

4 Typical CS Holdown at Floor
SCALE: 3/4"=1'-0"

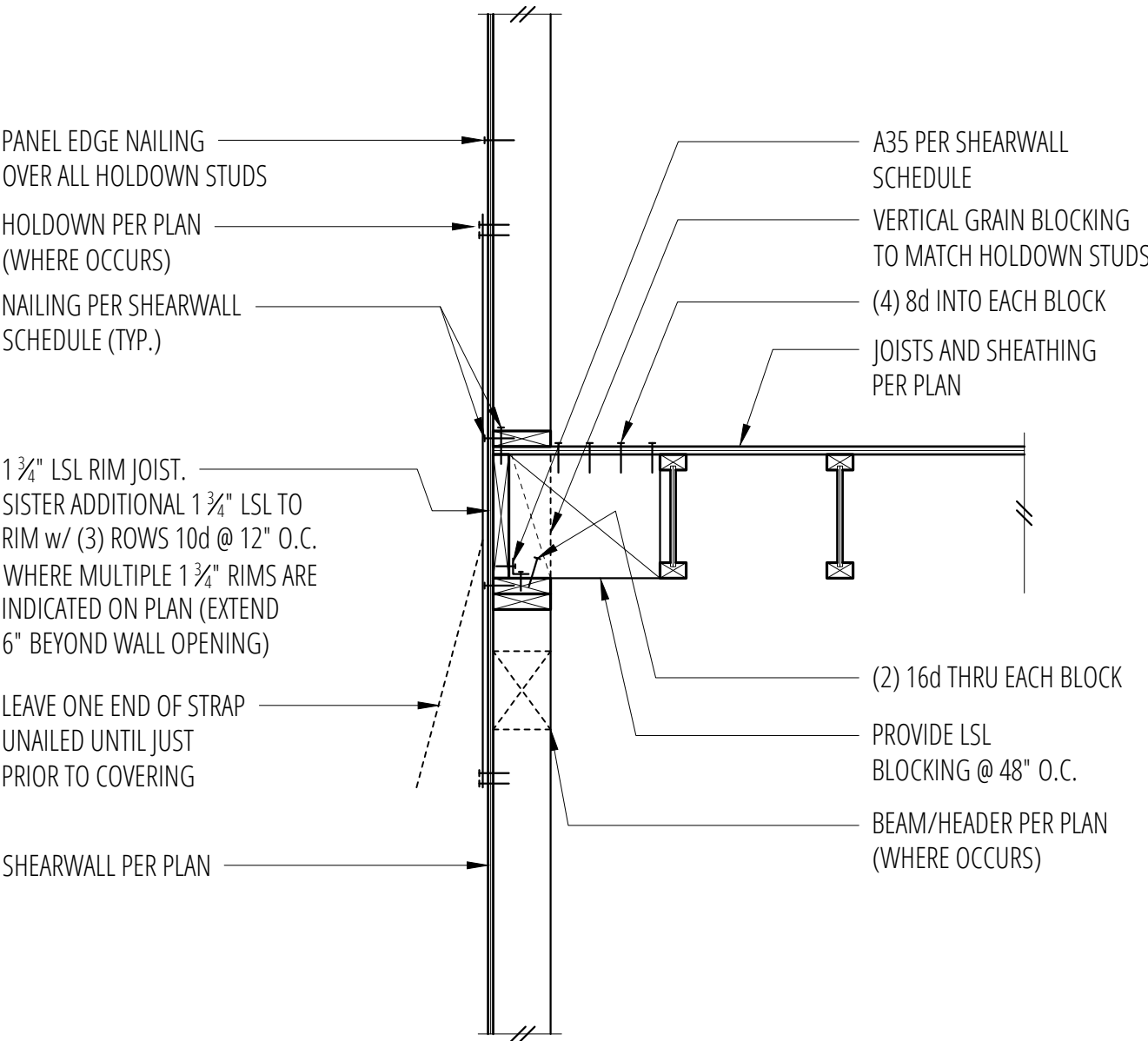


5 SCALE: 3/4"=1'-0"

6 SCALE: 3/4"=1'-0"

7 Floor Framing w/ Concrete Topping at Beam Connection
SCALE: 3/4"=1'-0"

8 Perpendicular Floor Framing (Continuous Rim)
SCALE: 3/4"=1'-0"



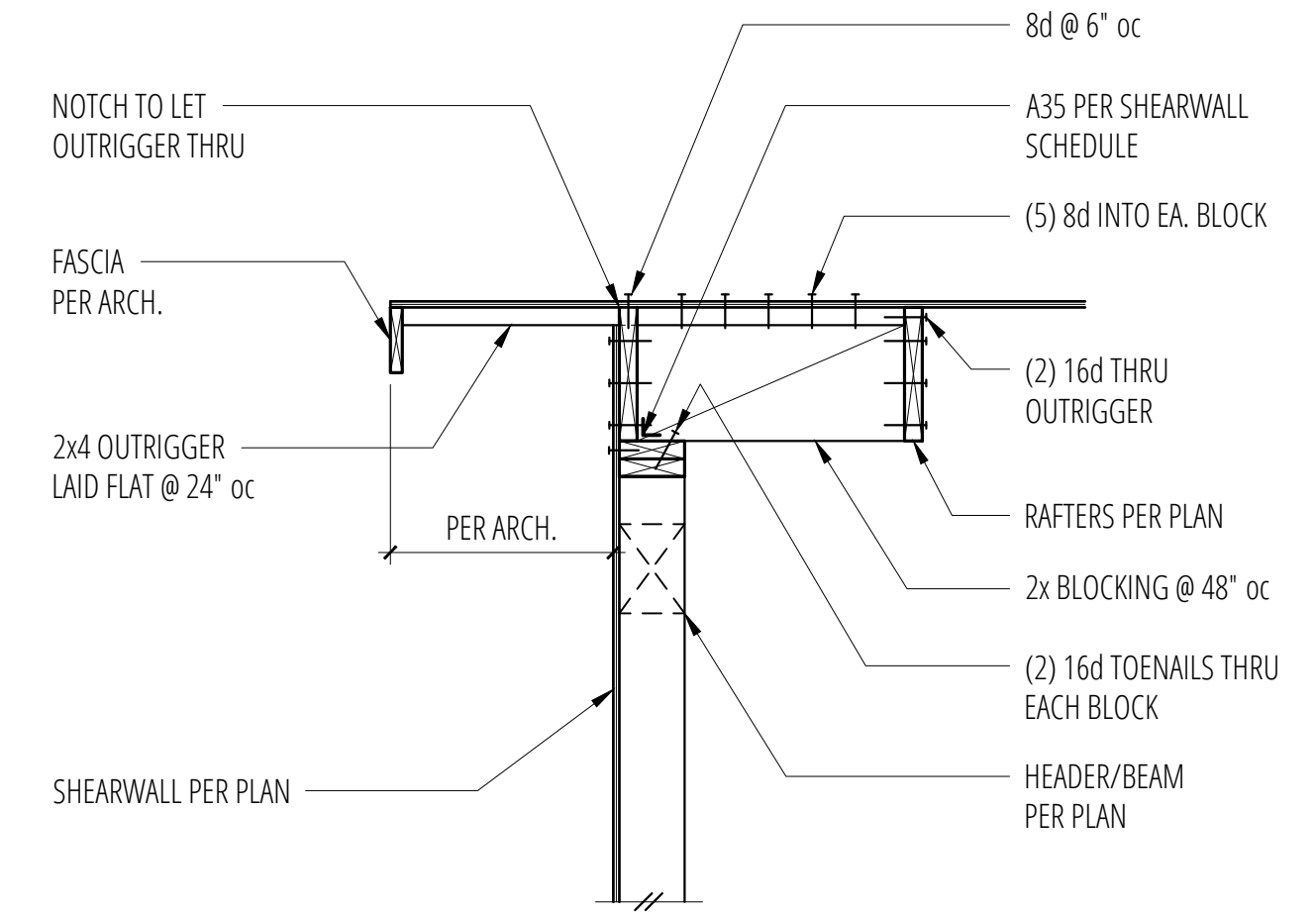
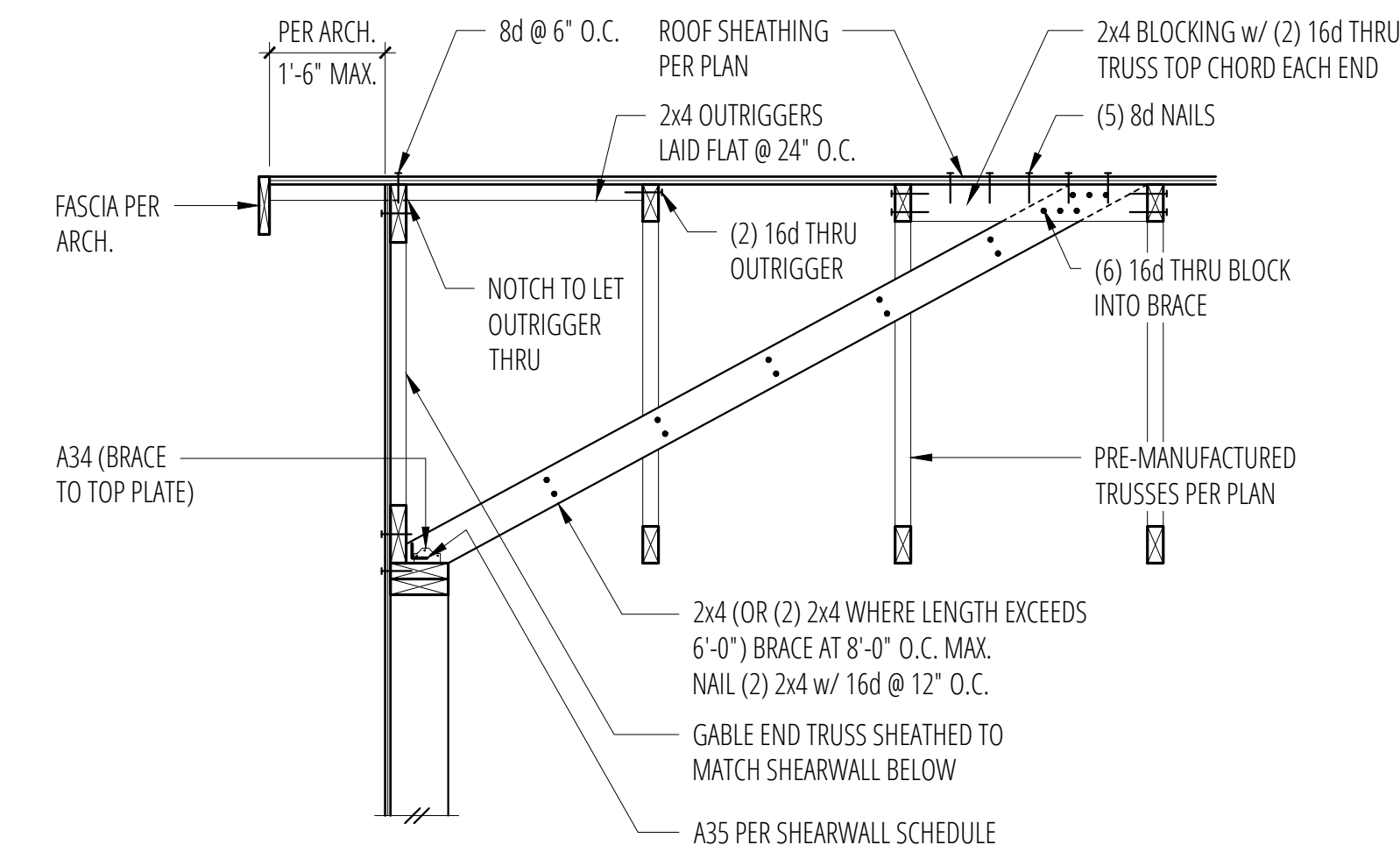
9 SCALE: 3/4"=1'-0"

10 SCALE: 3/4"=1'-0"

11 SCALE: 3/4"=1'-0"

12 Parallel Floor Framing (Continuous Rim)
SCALE: 3/4"=1'-0"

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FLOOR FRAMING DETAILS		
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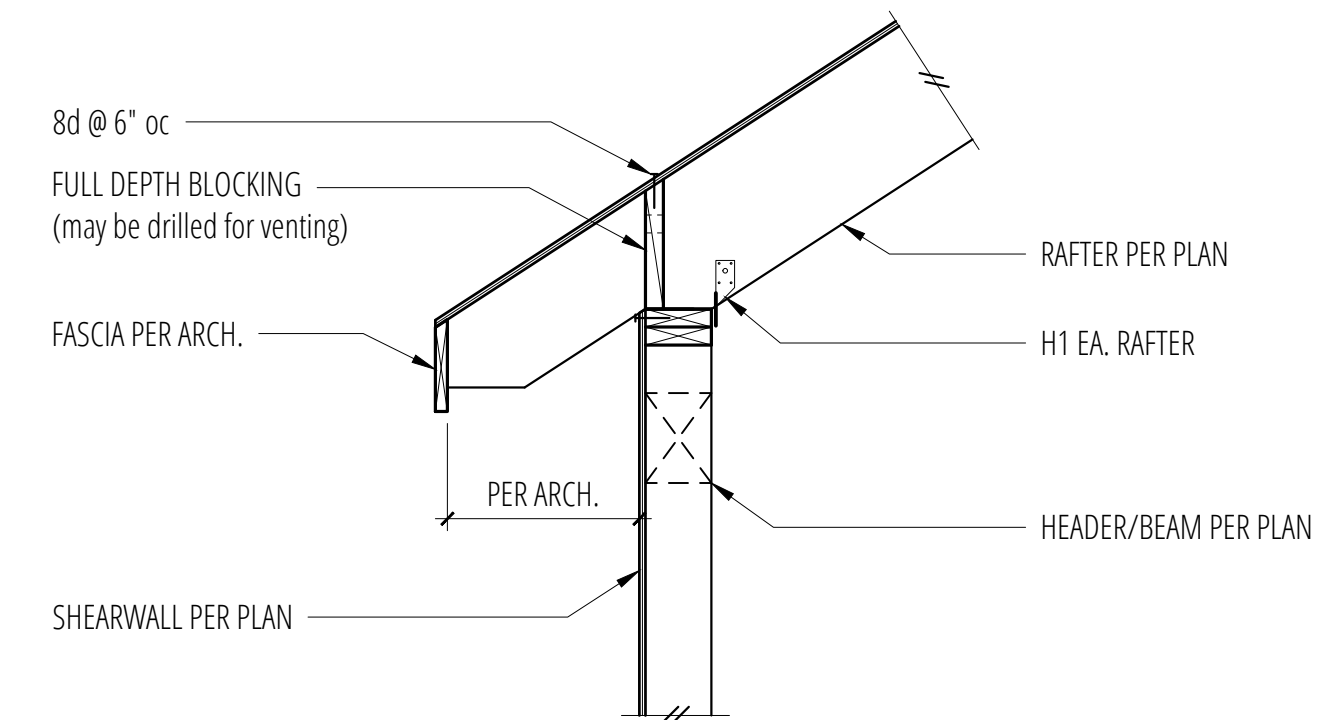
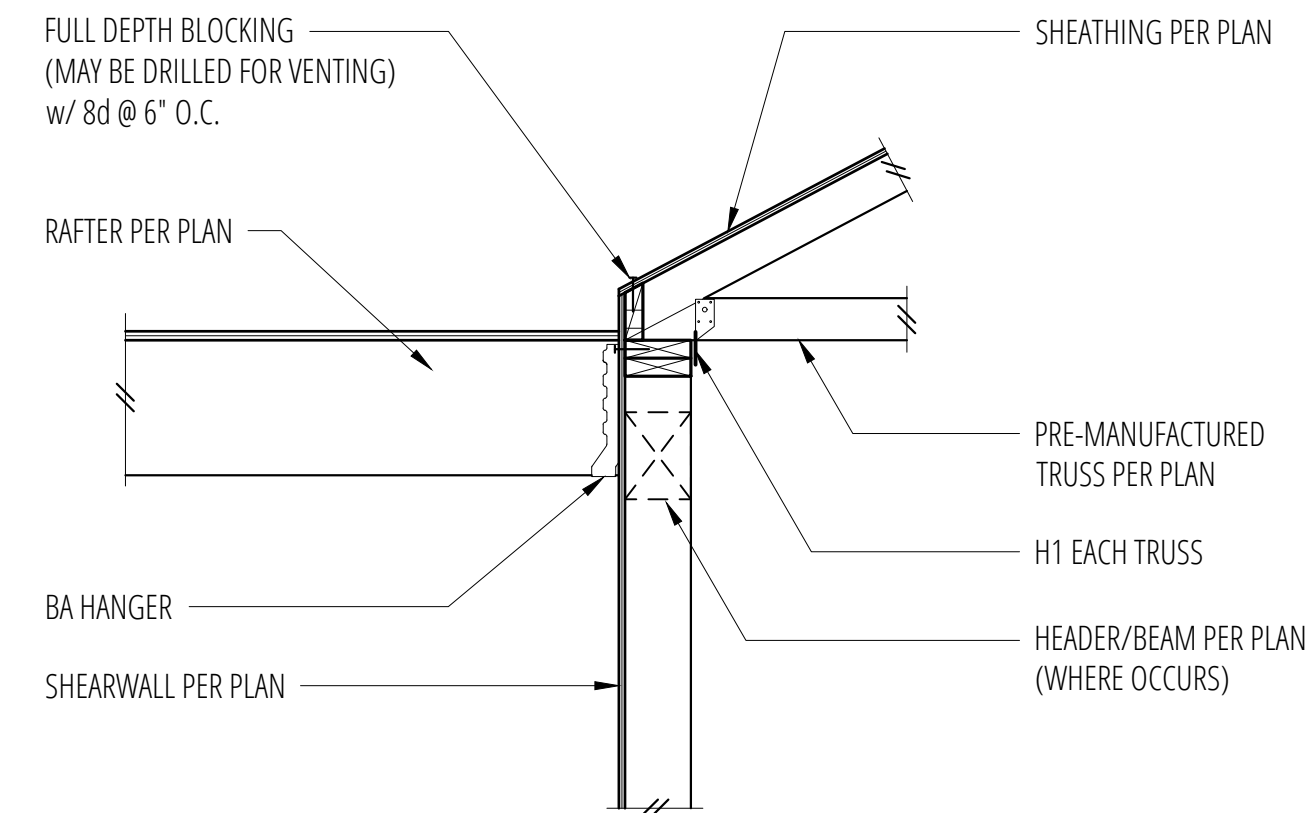


2 Overframing Connection

SCALE: 3/4"=1'-0"

3 Exterior Non-Bearing Wall at Roof (w/ Brace)
SCALE: 3/4"=1'-0"

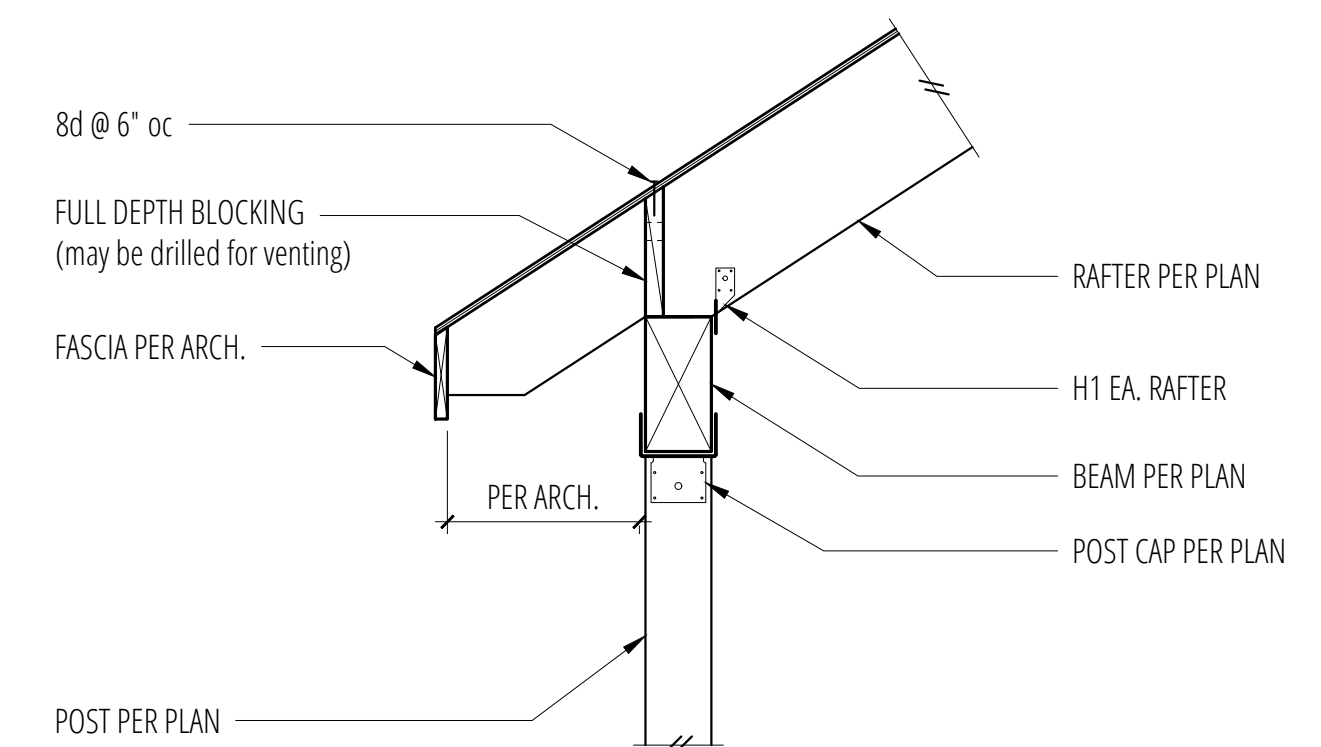
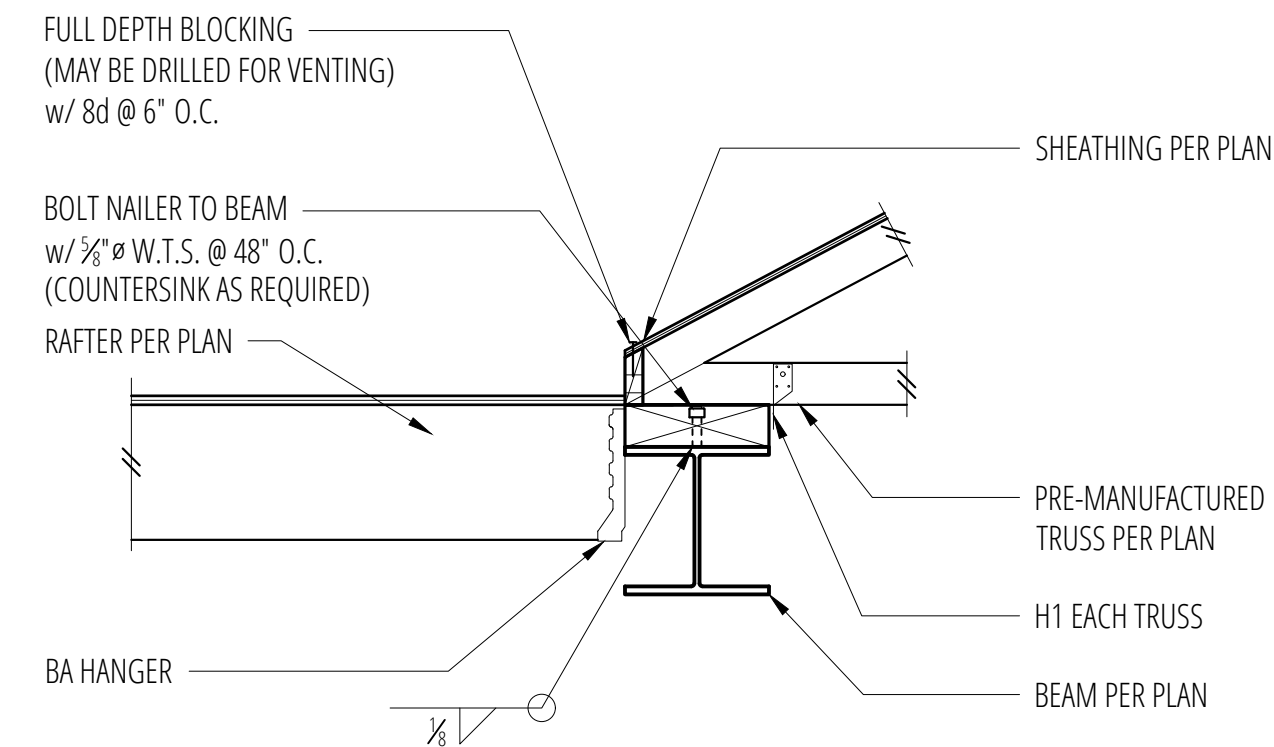
4 Exterior Non-Bearing Wall at Roof
SCALE: 3/4"=1'-0"



6 SCALE: 3/4"=1'-0"

7 Exterior Bearing Wall at Roof
SCALE: 3/4"=1'-0"

8 Exterior Bearing Wall At Roof
SCALE: 3/4"=1'-0"



10 SCALE: 3/4"=1'-0"

11 Beam & Post at Roof
SCALE: 3/4"=1'-0"

12 Beam & Post At Roof
SCALE: 3/4"=1'-0"

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ROOF FRAMING DETAILS	